

Concept and Parameters for Ground Improvement illustrated by case histories

by

СЕРГЕЙ ВАРАКСЙН

Serge VARAKSIN

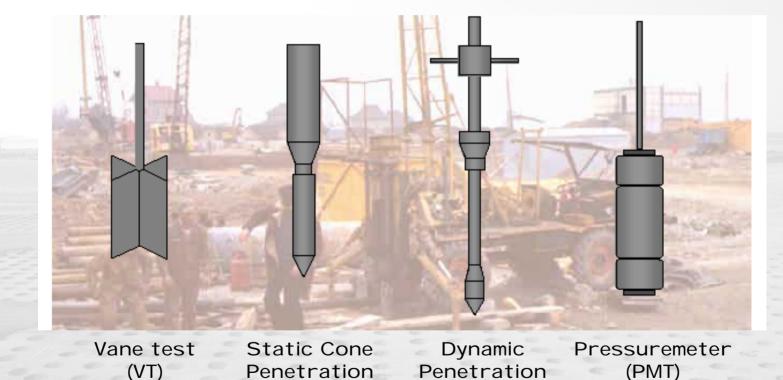
Chairman T.C. Ground Improvement (TC 211)

PARAMETERS RELATED TO GROUND IMPROVEMENT FROM IN SITU TESTS

Parameters related to ground improvement from in situ tests

- Menard Pressumeter (PMT)
- Static Cone Penetration (CPT)
- Dynamic Penetration (SPT)
- Vane Test (VT)
- Some correlations

Parameters related to ground improvement : Differents types of in situ tests



PERM - MASTER CLASS - NOV 2010

Test (SPT)

Test (CPT)

THE MENARD PRESSUREMETER

The Menard Pressuremeter : Typical loading tests





Typical *load tests* conducted on foundations : (i) PBT; and (ii) PMT (*not CPT or SPT*)

PBT – vertical load test

PMT – shear loading test

The Menard Pressuremeter : Stress – Srain curve of PMT results

MAIN PRESSURE GAUGE

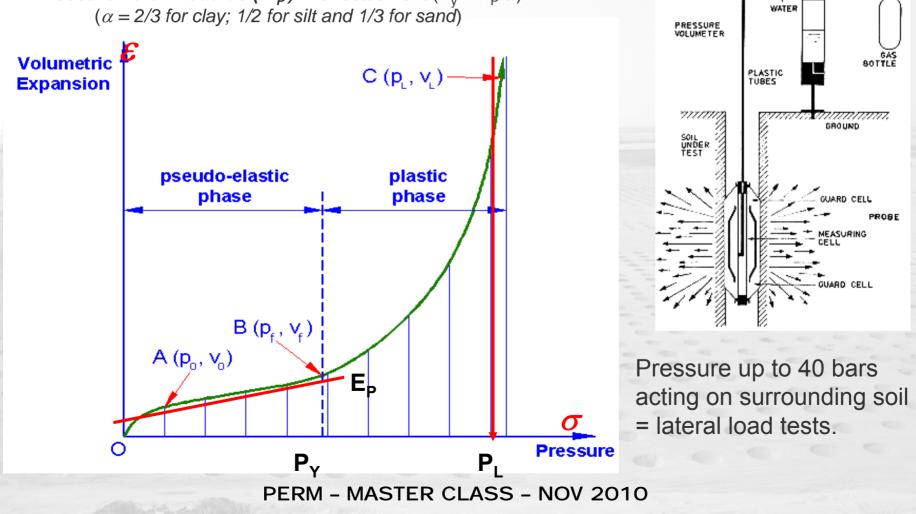
ALE

From the stress-strain (σ vs. ε) curve:

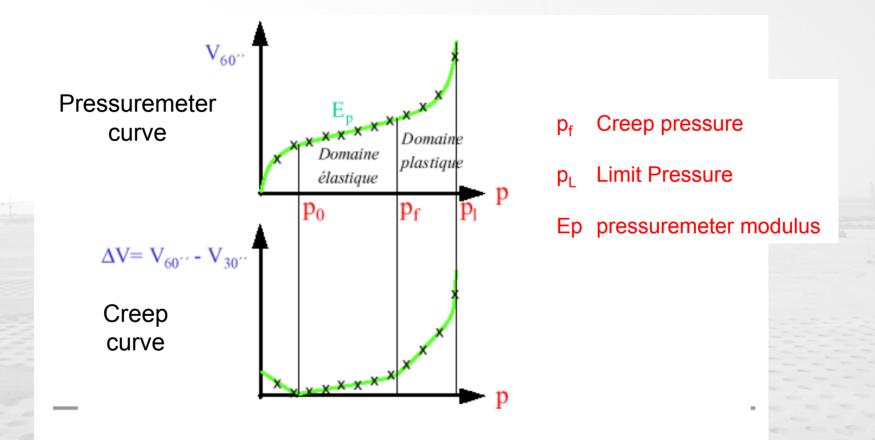
Limit Pressure (P₁) 1.

- for bearing capacity (= 5.5C_{...}).

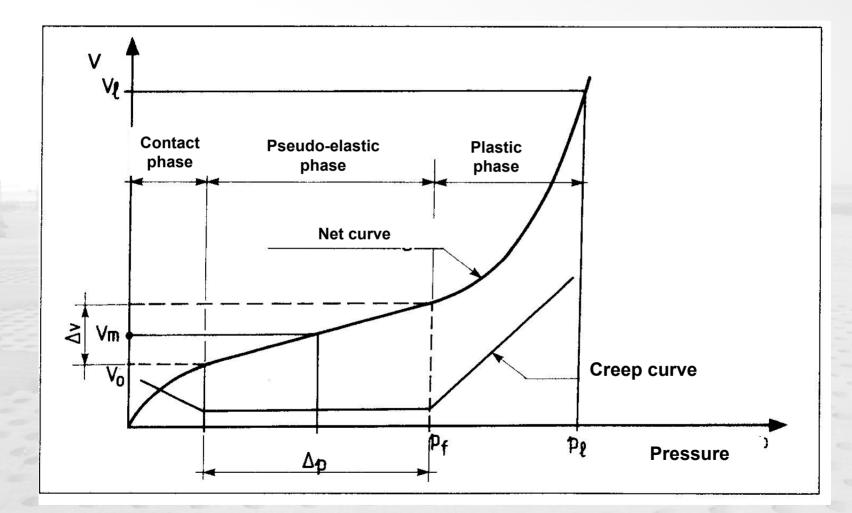
Pressuremeter Modulus (E_{P} **)** – for settlement ($E_{v} = E_{P}/\alpha$). 2. $(\alpha = 2/3 \text{ for clay}; 1/2 \text{ for silt and } 1/3 \text{ for sand})$



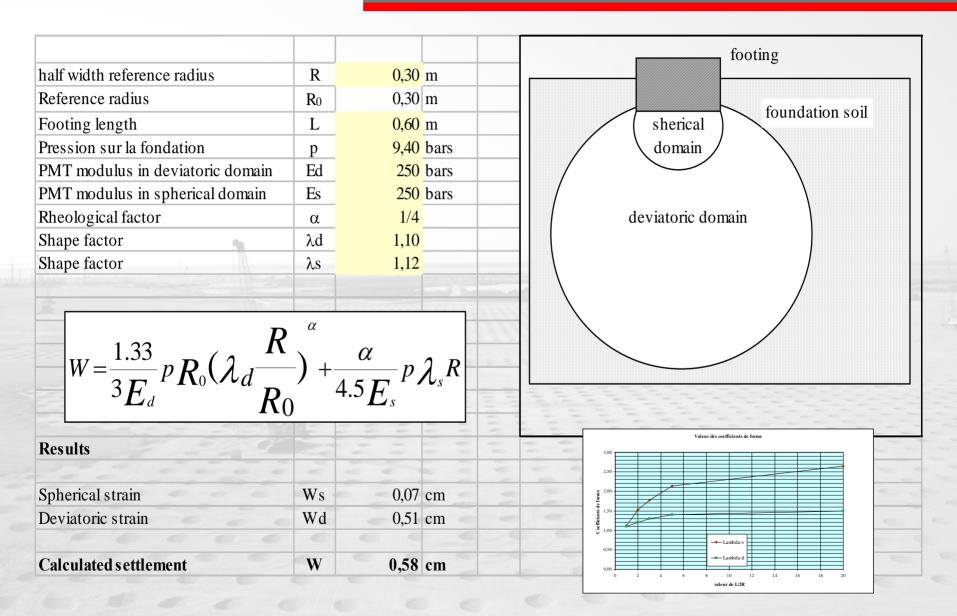
The Menard Pressuremeter



The Menard Pressuremeter : Net pressuremeter curve and creep curve

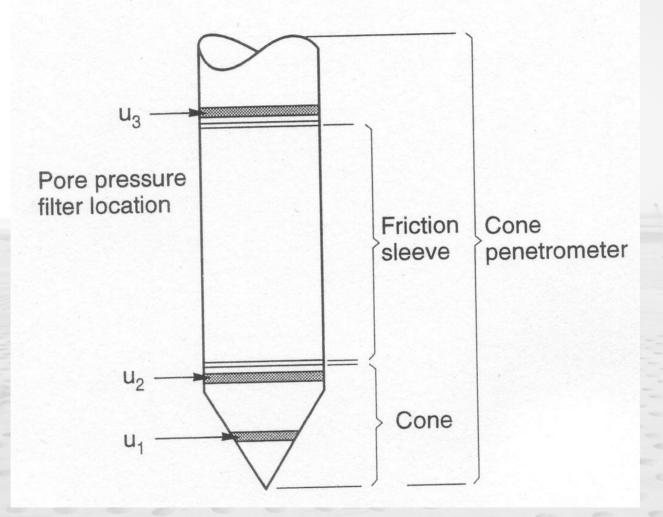


The Menard Pressuremeter : Settlement calculation under a footing

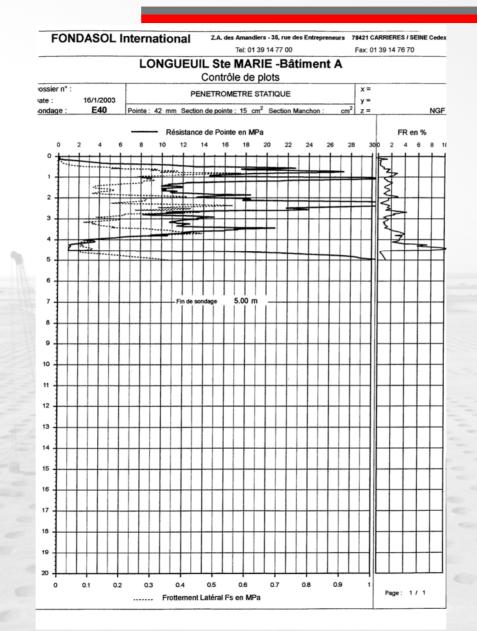


STATIC PENETRATION TEST (C.P.T.)

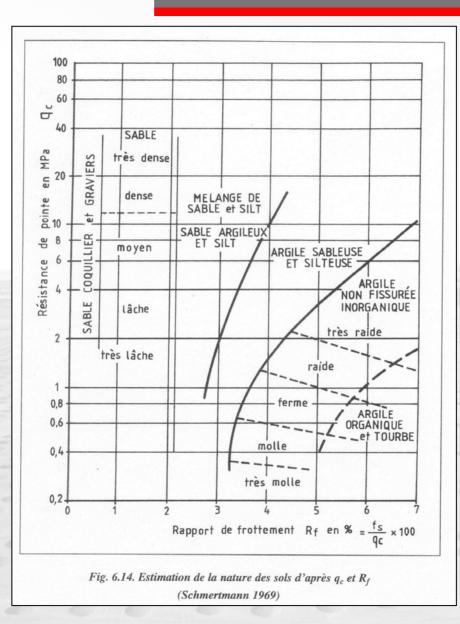
Static Penetration Test (C.P.T.)



Static Penetration Test : Typical CPT Test

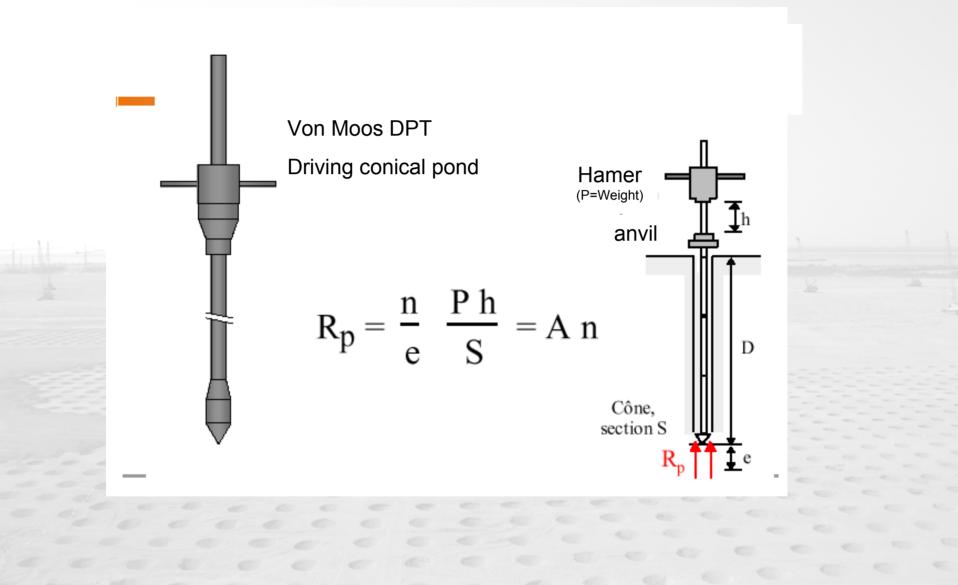


Static Penetration Test : Rought soil identification from CPT Test



DYNAMIC PENETRATION TEST (SPT, DPT)

Dynamic Penetration Test

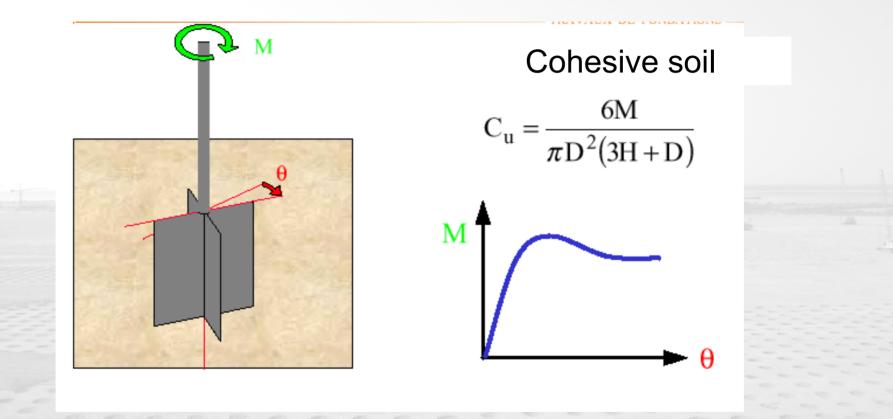


Dynamic Penetration Test



VANE TEST (only in soft homogeneous clay)

Vane Test



Undrained cohesion of soils

State of the Art Report



17TH International Conference on Soil Mechanics & Geotechnical Engineering

State of the Art Report

Construction Processes Procédés de Construction

> Jian Chu Nanyang Technological University, Singapore

> > Serge Varaksin

Menard, France

Ulrich Klotz

Zublin International GmbH, Germany

Patrick Mengé Dredging International n.v., DEME, Belgium



NOTA : TC 17 meeting ground improvement – 07/10/2009 Website : www.bbri.be/go/tc17

Category	Method	Principle
A. Ground improvement without admixtures in non-cohesive soils or fill materials	A1. Dynamic compaction	Densification of granular soil by dropping a heavy weight from air onto ground.
	A2. Vibrocompaction	Densification of granular soil using a vibratory probe inserted into ground.
	A3. Explosive compaction	Shock waves and vibrations are generated by blasting to cause granular soil ground to settle through liquefaction or compaction.
	A4. Electric pulse compaction	Densification of granular soil using the shock waves and energy generated by electric pulse under ultra-high voltage.
	A5. Surface compaction (including rapid impact compaction).	Compaction of fill or ground at the surface or shallow depth using a variety of compaction machines.
B. Ground improvement without admixtures in cohesive soils	B1. Replacement/displacement (including	Remove bad soil by excavation or displacement and replace it by good soil or rocks.
	load reduction using light weight materials)	Some light weight materials may be used as backfill to reduce the load or earth
		pressure.
	B2. Preloading using fill (including the use of vertical drains)	Fill is applied and removed to pre-consolidate compressible soil so that its compressibility will be much reduced when future loads are applied.
	B3. Preloading using vacuum (including combined fill and vacuum)	Vacuum pressure of up to 90 kPa is used to pre-consolidate compressible soil so that its compressibility will be much reduced when future loads are applied.
	B4. Dynamic consolidation with enhanced drainage (including the use of vacuum)	Similar to dynamic compaction except vertical or horizontal drains (or together with vacuum) are used to dissipate pore pressures generated in soil during compaction.
	B5. Electro-osmosis or electro-kinetic consolidation	DC current causes water in soil or solutions to flow from anodes to cathodes which are installed in soil.
	B6. Thermal stabilisation using heating or freezing	Change the physical or mechanical properties of soil permanently or temporarily by heating or freezing the soil.
	B7. Hydro-blasting compaction	Collapsible soil (loess) is compacted by a combined wetting and deep explosion action along a borehole.

C. Ground improvement with admixtures or inclusions	C1. Vibro replacement or stone columns	Hole jetted into soft, fine-grained soil and back filled with densely compacted gravel or sand to form columns.	
	C2. Dynamic replacement	Aggregates are driven into soil by high energy dynamic impact to form columns. The backfill can be either sand, gravel, stones or demolition debris.	
	C3. Sand compaction piles	Sand is fed into ground through a casing pipe and compacted by either vibration, dynamic impact, or static excitation to form columns.	
	C4. Geotextile confined columns	Sand is fed into a closed bottom geotextile lined cylindrical hole to form a column.	
L. Muur - hanne	C5. Rigid inclusions (or composite	Use of piles, rigid or semi-rigid bodies or columns which are either premade or formed	
-	foundation, also see Table 5)	in-situ to strengthen soft ground.	
-	C6. Geosynthetic reinforced column or pile	Use of piles, rigid or semi-rigid columns/inclusions and geosynthetic girds to enhance	
	supported embankment	the stability and reduce the settlement of embankments.	
	C7. Microbial methods	Use of microbial materials to modify soil to increase its strength or reduce its	
		permeability.	
	C8 Other methods	Unconventional methods, such as formation of sand piles using blasting and the use of bamboo, timber and other natural products.	

D. Ground improvement	D2. Chemical grouting	Solutions of two or more chemicals react in soil pores to form a gel or a solid precipitate to either increase the strength or reduce the permeability of soil or ground.	
with grouting D3. Mixing methods (including premixing or type admixtures deep mixing)		Treat the weak soil by mixing it with cement, lime, or other binders in-situ using a mixing machine or before placement	
51	D4. Jet grouting	High speed jets at depth erode the soil and inject grout to form columns or panels	
	D5. Compaction grouting	Very stiff, mortar-like grout is injected into discrete soil zones and remains in a	
		homogenous mass so as to densify loose soil or lift settled ground.	
	D6. Compensation grouting	Medium to high viscosity particulate suspensions is injected into the ground between	
and the second sec	and the second sec	a subsurface excavation and a structure in order to negate or reduce settlement of	
-		the structure due to ongoing excavation.	
	E1. Geosynthetics or mechanically stabilised	Use of the tensile strength of various steel or geosynthetic materials to enhance the	
E. Earth	earth (MSE)	shear strength of soil and stability of roads, foundations, embankments, slopes, or	
reinforcement		retaining walls.	
the second se	E2. Ground anchors or soil nails	Use of the tensile strength of embedded nails or anchors to enhance the stability of	
		slopes or retaining walls.	
5	E3. Biological methods using vegetation	Use of the roots of vegetation for stability of slopes.	

Laboratory Engineering Properties

Why Soil improvement?

- •To increase bearing capacity and stability (avoid failure)
- To reduce post construction settlements
- To reduce liquefaction risk (sismic area)

Advantages / classical solutions

- avoid deep foundation (price reduction also on structure work like slab on pile)
- avoid soil replacement
- save time
- •Avoid to change site
- •Save money !

Soil Improvement Techniques

	Without added materials	With added materials
Cohesive soil Peat , clay	1 Drainage 2 VAcuum	4 Dynamic replacement
· out , only in		5 Stone columns 6 CMC
Soil with friction	3 Dynamic	7 Jet Grouting 8 Cement Mixing
Sand , fill	consolidation 4 Vibroflottation	

Parameters For Concept

-Soil caracteristics

- -cohesive or non cohesive
- blocks ?
- Water content, water table position
- Organic materials
- -Soil thickness
- -Structure to support
 - -Isolated or uniform load -Deformability

-Site environment

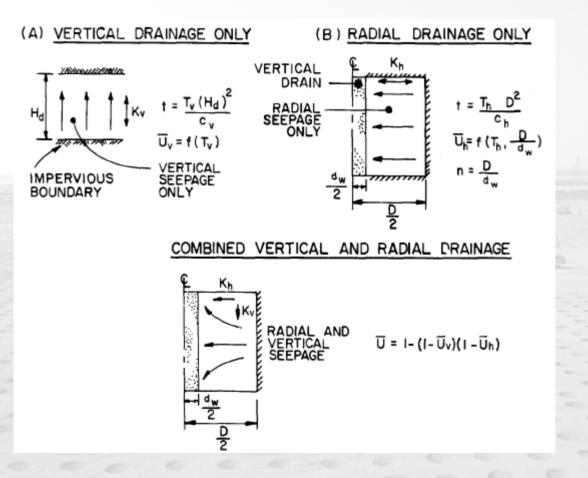
- -Close to existing structure
- -Height constraints
- -Time available to build

Preloading with vertical drains

σ=σ'+u

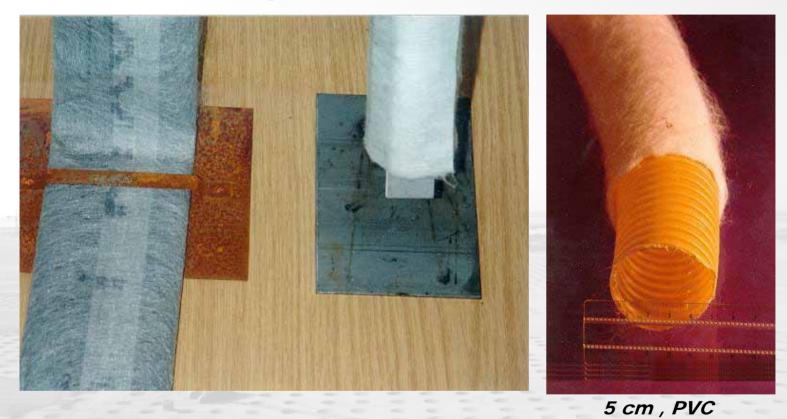
high fines contents soils

Radial and Vertical consolidation



Vertical drains: material

High fines contents soils

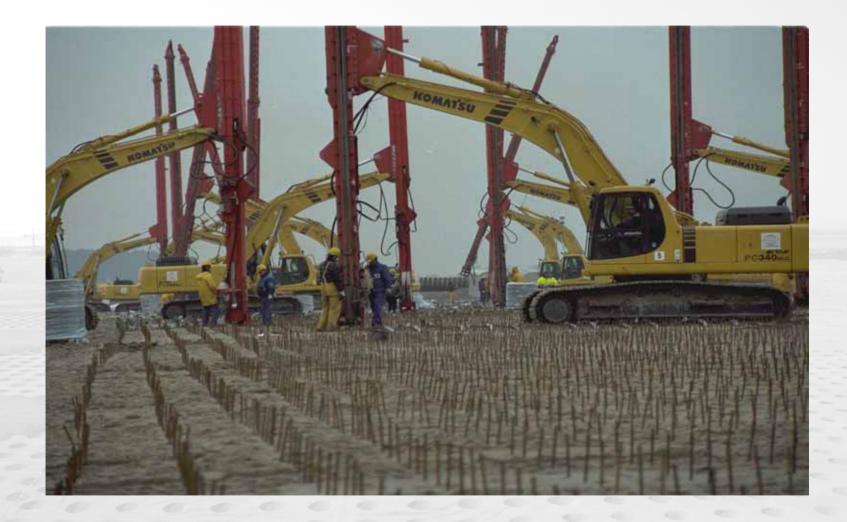


Flat drain

circular drain

vertical drain + geotextile

Vertical Drains



Vertical drains

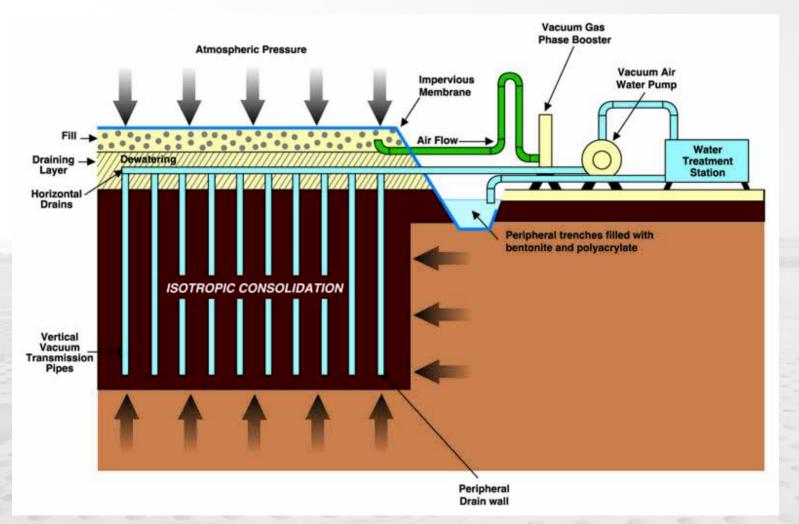
CONCEPT

- -Stable subsoil for surcharge
- -Soil can be penetrated
- -Time available is short
- -Some residual settlement is allowed

PARAMETERS

- 1 Depth
- 2 Drainage path
- 3 Cohesion
- 4 Consolidation parameters (oedometer, CPT) $e_0, C_C, C_V, C_R, C_\alpha, t,$ CPT dissipation test

Vacuum Consolidation (high fines contents soils)



VACUUM (J.M. COGNON PATENT)

Vacuum Consolidation

CONCEPT

- -Soil is too soft for surcharge
- -Time does not allow for step loading
- -Surcharge soil not available
- -Available area does not allow for berns

PARAMETERS

- 1 Depth
- 2 Drainage path
- 3 Condition of impervious soil
- 4 Watertable near surface
- 5 Absence of pervious continuous layer
- 6 Cohesion
- 7 Consolidation parameters (oedometer, CPT)
 - e_{O} , C_{C} , C_{V} , C_{R} , C_{α} , t, CPT dissipation test
- 8 Theoretical depression value
- 9 Field coefficient vacuum
- 10 Reach consolidation to effective pressure in every layer
- 11 Target approach

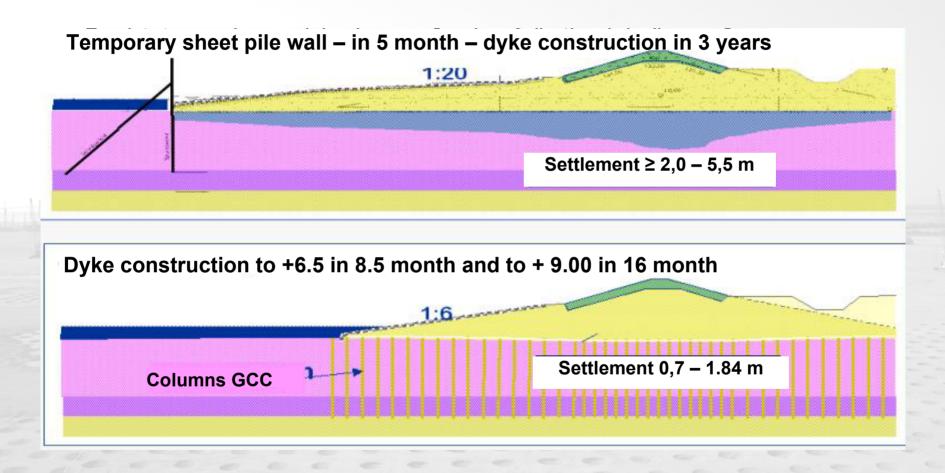


Case history – EADS Airbus Plant, Hamburg

General overview of Airbus site

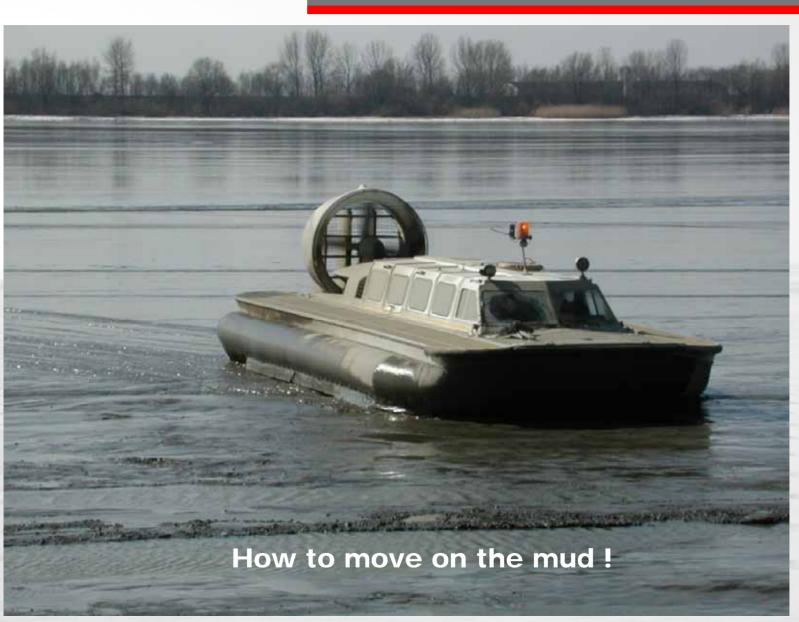


Basic design and alternate concept of Moebius–Menard



Subsoil characteristics

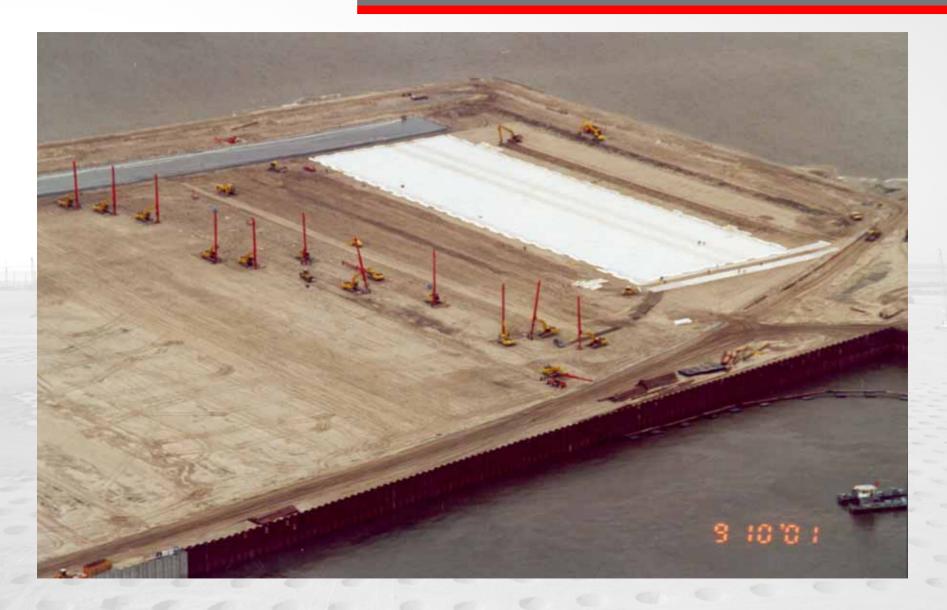
Soil type	Water content	Density	Shear strength		strength Modulus (under σ _z =		Deformation Modulus (under σ_z = 100 kN/m ²)	Coefficient of consolidation	Coefficient of secondary consolidation
	W (%)	γ/γ' – kN/m ³	δ'(°)/c' (kN/m²)	C _u (kN/m²)	E _S (MN/m²)	C _V (m²/year)	Cα (-)		
Mud	142	13/3	20/0	0.5-5	0.8	0.35	0.03		
Young clay	119	14/4	20/0	2-10	0.9	0.35	0.03		
Clay	70	15/5	17.5/10	5-20	1.5	0.5	0.02		
Peaty clay	139	14/4	20/5	5-20	0.9	0.4	0.03		
Peat	240	11/1	20/0	5-15	0.5	≥ 0.4	0.04		



Case history – EADS Airbus Plant, Hamburg



Case history – EADS Airbus Plant, Hamburg





PROJECT OVERVIEW





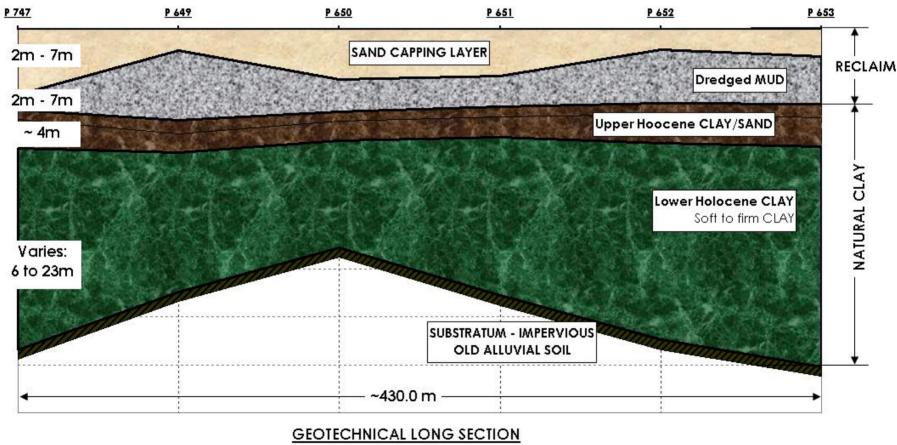
Located at the mouth of the Brisbane river; New reclamation area: 234 ha enclose

234 ha enclosed in the Port Expansion Seawall;

Part of the new reclaimed area to be ready in 5years; Seawall construction completed in 2005;



GEOTECHNICAL LONG SECTION



SCALE N.T.S

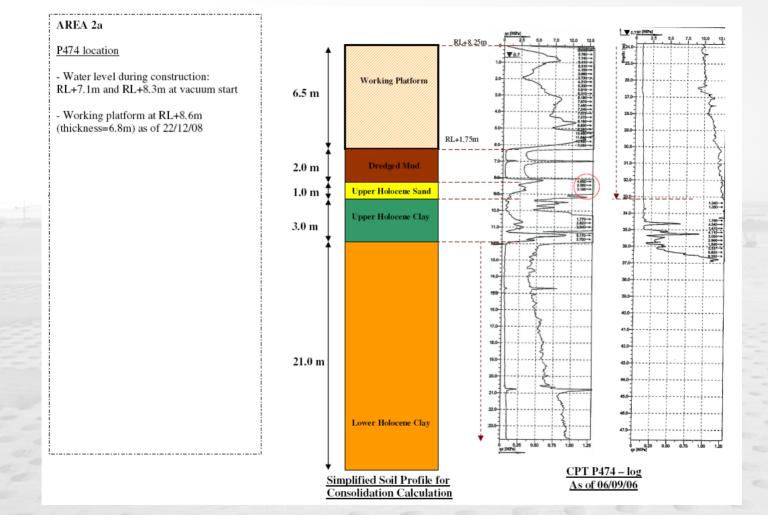


GEOLOGICAL PARAMETERS

Parameter	Unit	Dredged Material	Upper Holocene Sand	Upper Holocene Clay	Lower Holocene Clay
C _c /(1+e ₀)	[-]	0,235	0,01	0,18	0,235
C _α /(1+e ₀)	[-]	0,0059	0,001	0,008	0,0076
γ	[kN/m ³]	14	19	16	16
C _v	[m²/y]	1	10	10	0.9
C _h	[m²/y]	1	10	10	1.8
Su	[kPa]	4		20	28
S _u / σ' _v	- E]	0,25	0,3	0,3	0,2



GEOLOGICAL SOIL PROFILE





DESIGN CRITERIA & ASSUMPTIONS

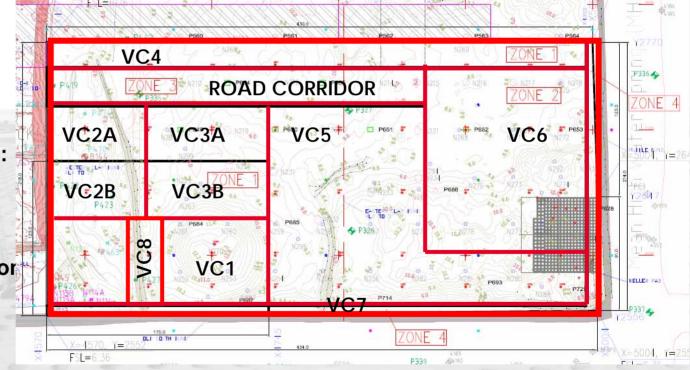
Service Load:

36kPa
25kPa
15kPa
5kPa

Residual Settlement (20y): Zone 1 to 3: 150mm Zone 4: 300mm

Vacuum pumping operation 18 months

Vacuum depressure: 75.0 kPa







DESK STUDY – NUMERICAL ANALYSIS USING EXCEL SPREADSHEET SETTLEMENT CALC.XLC

Calculation of *primary and secondary* settlement;

•Secondary settlement to commence after primary settlement;

•Change in vertical stress is constant over the depth of the stratum;

• **Buoyancy effect** on the fill below the groundwater level due to settlement

•Fill to be removed instantaneous at the end of preloading period;

•Design load immediately applied at end of preloading period;

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LOAD PARAMETERS Parameter Fill-bandly Grafter (Fill-bi-disor Taxon athorse files: Asset)	Home of Unit Unit	(ruge	5 Hard Her 6.979 5	074.80 074.80 2542 1430 1.97 0.443 0.443	WF rend 798.00 24482 1.400 7.990 8.002	197.00 2144 4 5.600 7.994 8.047 4	Final F Adv Terror 40.00 20:45 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.00000 5.00000 5.00000 5.00000 5.00000 5.00000 5.000000 5.0000000000	Flatfarm fi a fand (17a 21.0 21.0 21.0 5.0 6.0 6.0 0	1. Cm2 43 (1/m23) 47.00 (544 7 1.400 0.700 0.000	1.715 0.575 7.570 42.95 2.520 2.520 0.716 0.716 0.000		1.00	1.00	8.00	0.00	8.09	8.00	1.01	8.00	
LOAD PARAMETERS Personalise 20:156 20:156 Second of the Advance Terrors and the Advance Second of the Advanc	Home of Unit Unit	(ruge	5	074.80 074.80 2542 1430 1.97 0.443 0.443	WF rend 198.00 1440 7,110 8,002 10000000	197.00 2144 4 5.600 7.994 8.047 4	Final F Line Versum 45.00 2545 5.000 1.700 6.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.00000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000 5.00000 5.0000 5.00000 5.00000000	Flatfarm Fr a land (17a Francestar 2008 1 2008 1 0.000 0.000 0.000 0.000	11. Cm2 12. 147-22) 147-22 147-22 147-22 14.00 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.0000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.000	4.714 9.540 7.540 2.400 2.400 2.400 9.714 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.040 9.0400 9.0400 9.0400 9.040000000000		1.00	1.00		0.00	1.01	5.00		8.00	•
LOAD PARAMETERS Parameter Fill-bandly Grafter (Fill-bi-disor Taxon athorse files: Asset)	Home of Unit Unit	(ruge	Dru Kapi d Han 8,979 25,421 4,719 8,865 4,719 8,865 5,899 5,899	VP, and STR 20 3542 3542 3542 3542 3542 3542 3542 3542	697 cm4 T96.00 25402 1.00 7,00 8,00 9 10/30/200 8,200	87.00 25.00 4 5.000 7.100 8.007 0 5.002000 8.200	Final F Nerven Farren 8000 5.700 6.000 5.700 6.000 5.700 6.000	Plandara R a Inné Cita Prana rian A 2000 2544 5544 5544 5544 5544 5544 5544	11. (m) 12. (17m2) 14.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15	4.711 9.500 7.500 2.400 ct.00 2.500 2.500 9.700 9.700 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.000000 9.0000 9.00000000										
LOAD PARAMETERS Parameter 2015s 2015s Description of the disease Terror subtrate filter Assets 2016 of the disease of the	Home of Unit Unit 	f stage Nymboll F	Ever Hay 414a 6.579 23ag 1 4.299 8.384 9 9 9.990 9.090 9.090	97 and 974 FF 354 2 3 4 5 FF 3 4 4 5 9 104 FF 4 5 FF 4 5 FF 4 5 FF 4 5 FF	140 rend 1448 2 1,000 1,000 1,000 1000/000 8,200 4,200	87.00 25.00 4 5.000 7.100 8.007 0 5.002000 8.200	Final F Line Versum 45.00 2545 5.000 1.700 6.000 5.000 5.000 5.000 5.000	Plandara R a Inné Cita Prana rian A 2000 2544 5544 5544 5544 5544 5544 5544	11. (m) 12. (17m2) 14.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15.00 15	4.711 9.500 7.500 2.400 ct.00 2.500 2.500 9.700 9.700 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.0000 9.000000 9.0000 9.00000000		1.00	6.00	8.00	5.00	8.00	1.00	5.00	1.00	
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- •Up to 15 surcharge steps;
- •Up to 30 soil layers;
- •Calculation of *shear strength increase* during consolidation of cohesive soils;

DESK STUDY...

- •Different types of *drains* available: MCD 34, MD88-3, FD767;
- Effect of smear due to mandrel insertion
- •Graphical output Settlement / Fill thickness chart

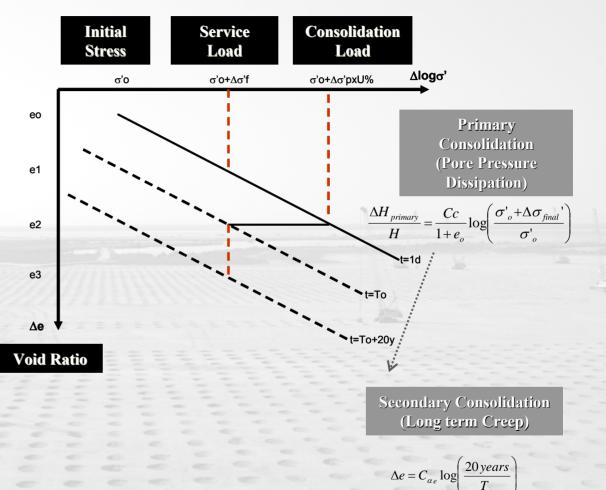
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For energy of the second secon	Usit Mm) - - - - - - - - - - - - - - - - - - -	Syntat 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	8.07 2.02 4.00 8.00 7.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00 8.00	0174.00 1542 1.550 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00 7.	794.00 7942 1.400 7942 7940 7940 7940 7940 7940 7940 7940 7940	87.00 25.07 7.00 7.00 8.00 7.00 7.00 7.00 7.00 4.00 4.00 4.00 4	Parton Briton 3545 1545 450 450 450 450 450 450 450 450 450	0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000	(Jal) Provide 10 25x 7 1 20 37x 7	1921 24.59 42.59 29.59 29.59 39.59 4.000 9.700 9.700 9.700	-4199.00 + 200 0.024 0.020 0.020	9.700 9.400 9.934 9.899	6.700 0.400 9.024 8.609	6.000 6.000 6.024 8.000	4.200 4.200 4.224 8.000 4	4.300 4.000 4.524 8.000 6	0.000 0.000 0.024 0.000	+.700 0.000 0.024 0.000	+.700 +.700 +.724 +.000	8.3 8.4 9.6 8.4
The ender of the second	Usit Mm) - - - - - - - - - - - - - - - - - - -	Synthet I I far fill r Veise ise Synthet f da da da b	8.07 2.07 2.07 4.70 4.70 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 5.400 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ANALYSIS METHOD

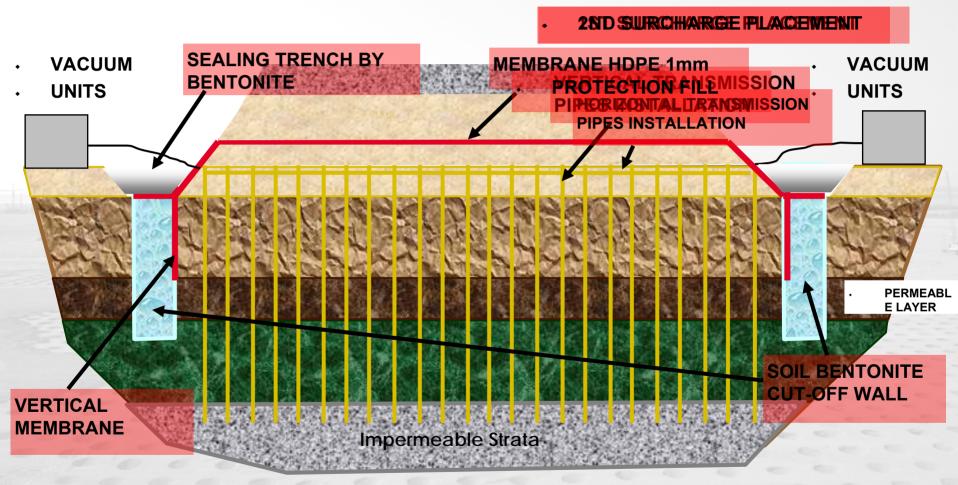
•Secondary Settlement Program uses a method based on Bjerrum's concept to calculate instantaneous and delayed consolidation (Bjerrum, 1967).



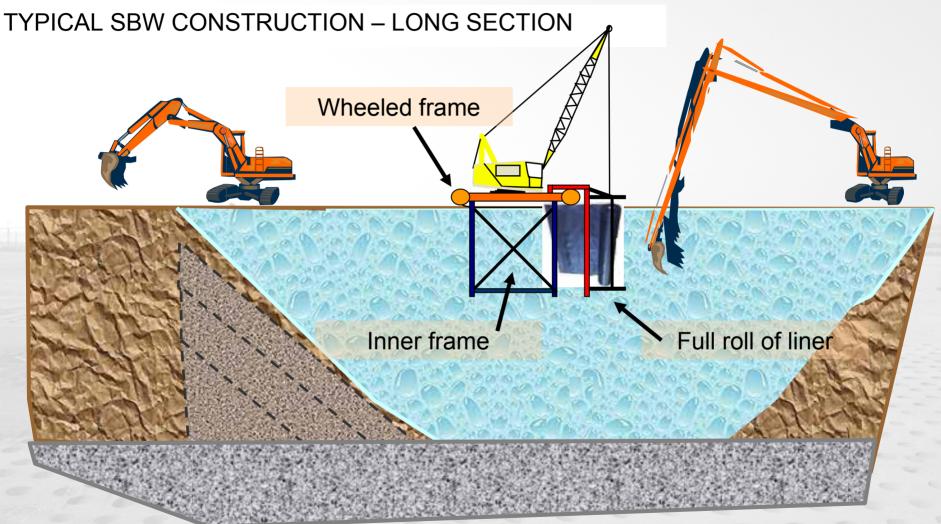




CONSTRUCTION SEQUENCE



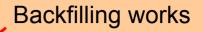












Two membrane rolls - overlapping **Trench** excavation works under bentonite slurry







Project: Vacuum Consolidation of Paddock S3B Proj. No.: 5040101 Section: RR19

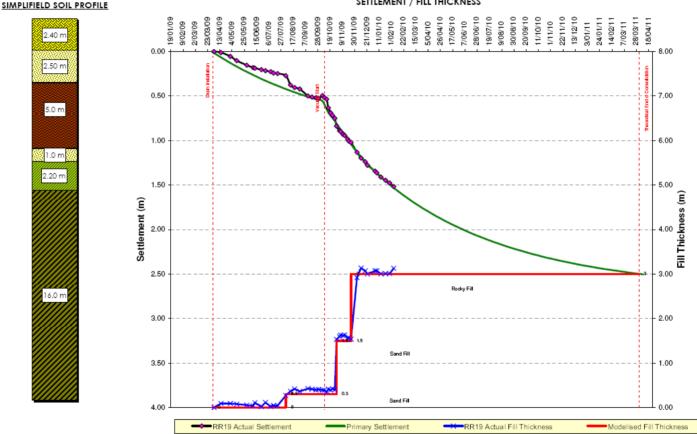
2.40 m

2.50 m

5.0 m

1.0 m 2.20 m

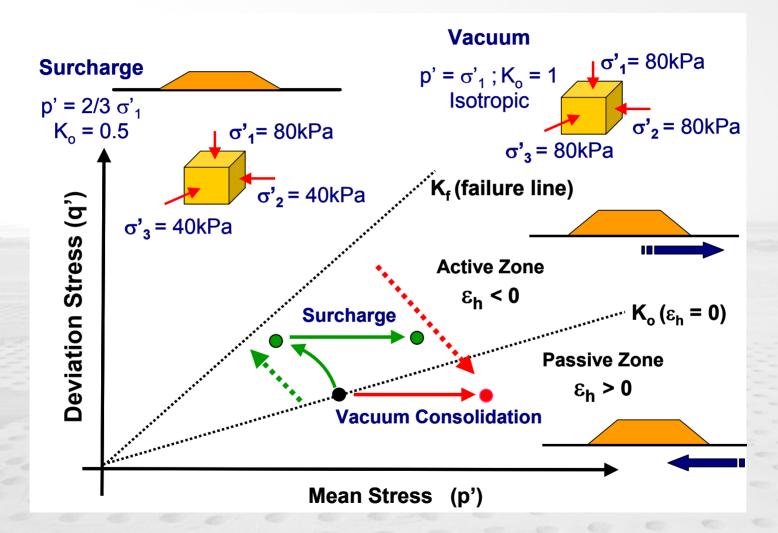
16.0 r



SETTLEMENT / FILL THICKNESS

WATER LEVEL: R.L. +7.10 DURING PUMPING: R.L. +8.80 WORKING PLATFORM: R.L. +9.00 SOIL PROFILE: CPT: P651

Stress path for Vacuum Process



Case history : Kimhae (Korea) - 1998

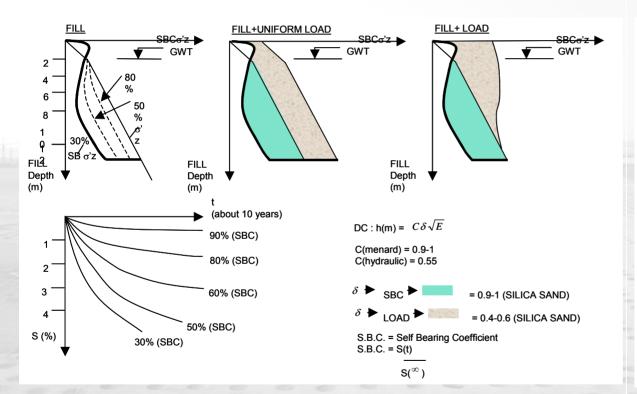


Soil Improvement Techniques

	Without added materials	With added materials
Cohesive soil Peat , clay	1 Drainage 2 VAcuum	4 Dynamic replacement
r out , oldy		5 Stone columns 6 CMC 7 Jet Grouting
Soil with friction	3 Dynamic consolidation 4 Vibroflottation	8 Cement Mixing
Sand , fill		

Parameters for Concept

CONCEPT



PARAMETERS

- Age if fill saturated or not - P_L -Selfbearing level - \emptyset - E_P or E_M - Q_C , F_R , -N-R.D. (???) -Shear wave velocity -Seismic parameters -Grain size

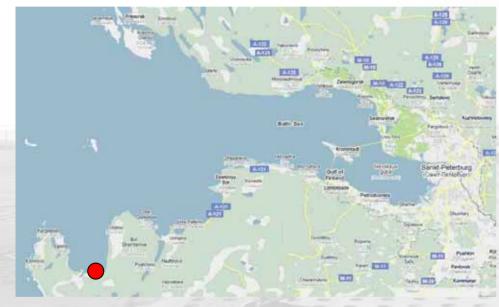
Case History

Nice airport runway consolidation Granular soil



Very high energy (250 t , 40 m)

Site localisation - Место объекта



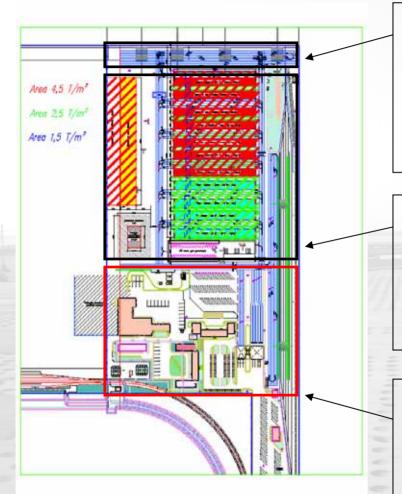
- 150 km from Saint-Petersburg
 (150 км от Санкт-Петербурга)
- Main Port development program
 on the Baltic Sea

(Самый важний объект порта на Балтиском море)

Site overview - Вид объекта



Plan view - схема расположения

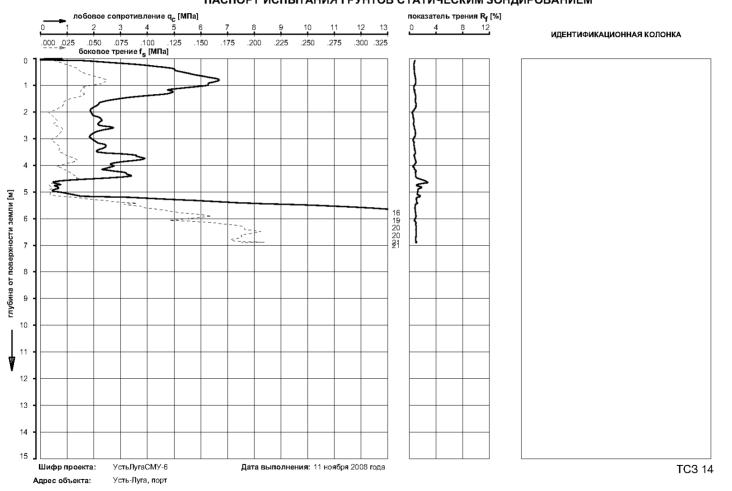


- heavy loads (тяжелая нагрузка)
- 6m of sand fill (6м песка)
- along the quay wall (вдоль причала)
- => Vibroflotation (виброфлотация)
- heavy loads (тяжелая нагрузка)
- 3 to 6m of sand fill (от 3 до 6м песка)
- => Dynamic Compaction (Динамическое Уплотнение)
- light loads (малинкая нагрузка)
- less than 3m of sand fill (меньшее 3м песка)
- => No compaction foreseen (ничего)

Few pictures before works (несколько картини до работы)



Initial soil conditions : C.P.T. (геологическая условия до работы)



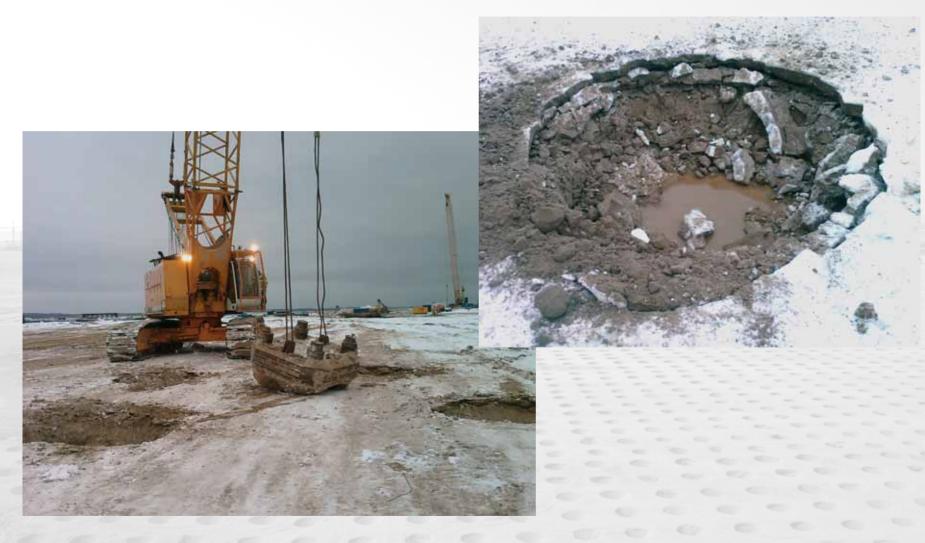
ПАСПОРТ ИСПЫТАНИЯ ГРУНТОВ СТАТИЧЕСКИМ ЗОНДИРОВАНИЕМ

ООО НТП "Фугро Геостатика"

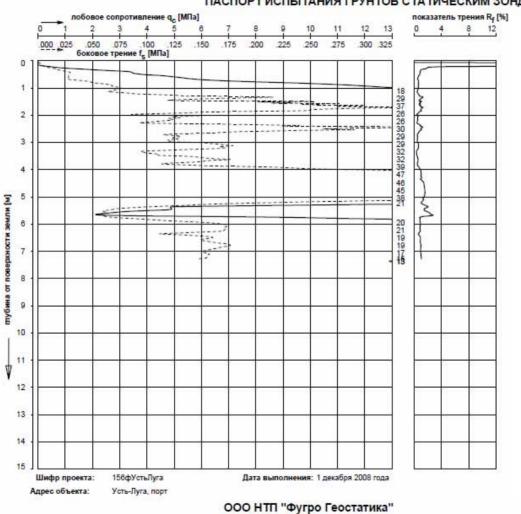
Few pictures of the ongoing works : Vibroflotation (Картини от Виброфлотации)



Few pictures of the ongoing works : DC (Картини от ДУ)

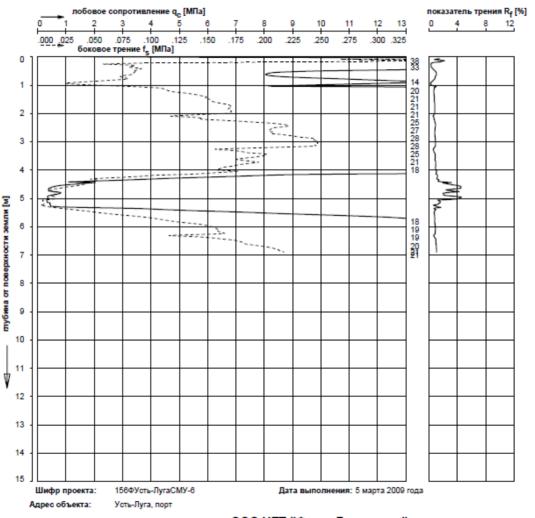


Results after compaction : Vibroflotation (Результат после Виброфлотации)



ПАСПОРТ ИСПЫТАНИЯ ГРУНТОВ СТАТИЧЕСКИМ ЗОНДИРОВАНИЕМ

Results after compaction : Dynamic Compaction (Результат после Динамического Уплотнения)



ПАСПОРТ ИСПЫТАНИЯ ГРУНТОВ СТАТИЧЕСКИМ ЗОНДИРОВАНИЕМ

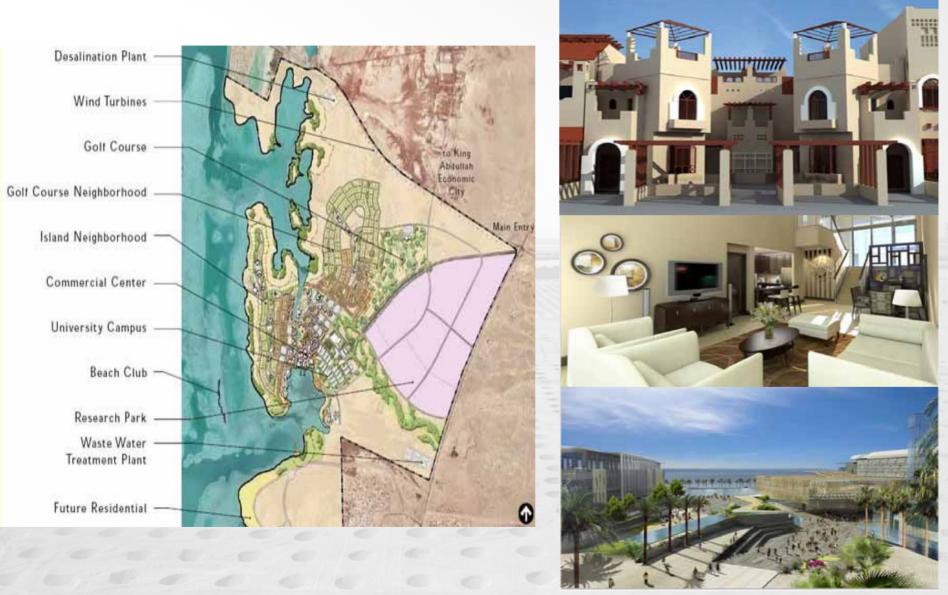
ООО НТП "Фугро Геостатика"

KAUST PROJECT

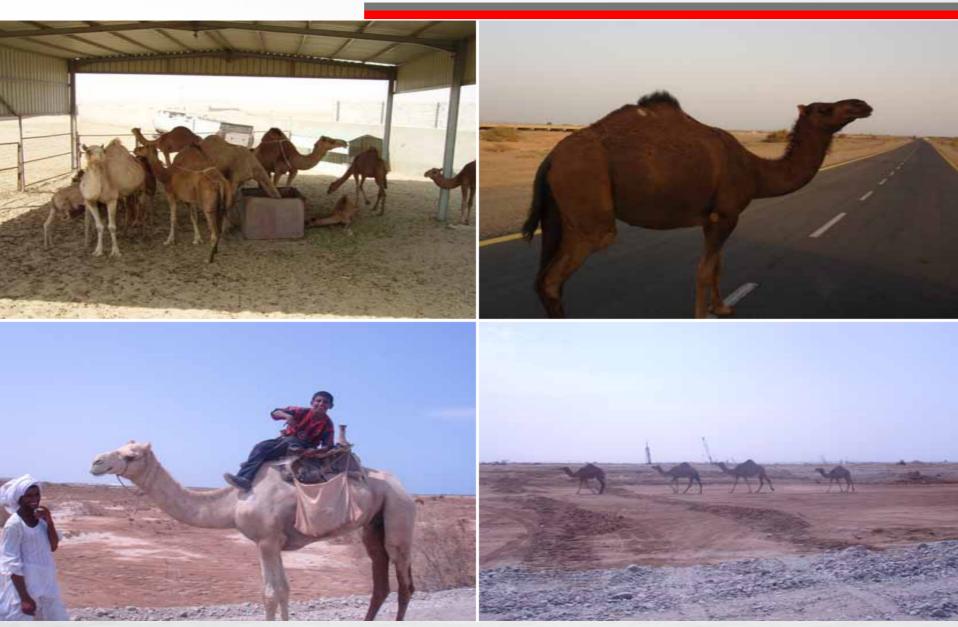
Concept and application of ground improvement for a 2,600,000 m²



Typical Master Plan



Discovering the Habitants



Areas to be treated

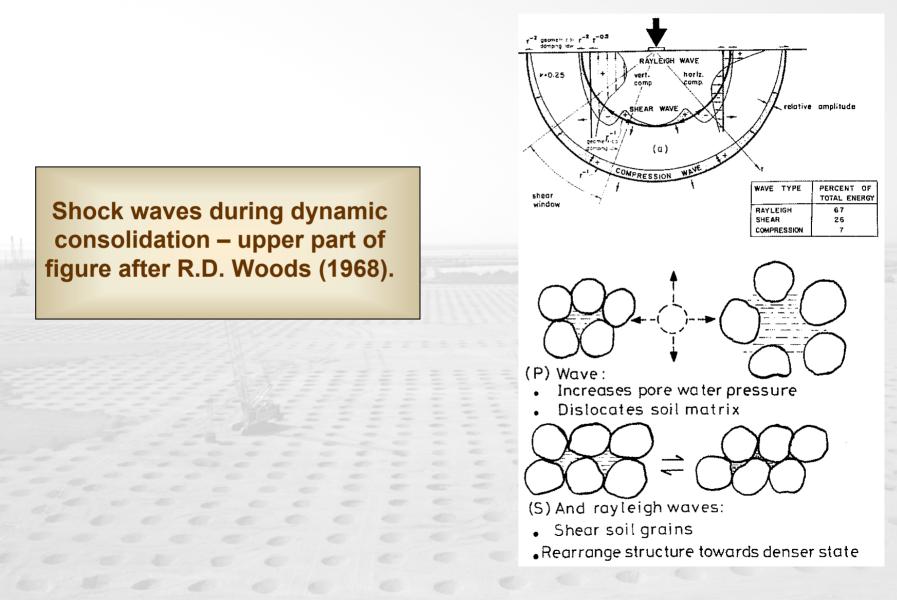
AREAS TO BE TREATED

•AL KHODARI (1.800.000 m2) •BIN LADIN (720.000 m2)

SCHEDULE

• 8 month

Dynamic Consolidation



Specifications

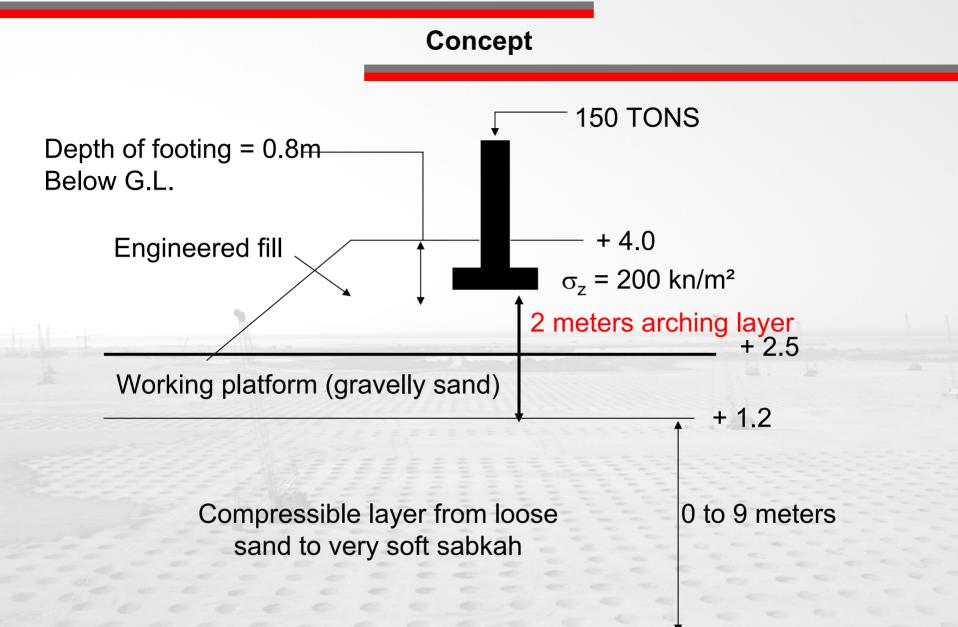
Isolated footings up to 150 tons

•Bearing capacity 200 kPa

•Maximum footing settlement 25 mm

Maximum differential settlement 1/500

Footing location unknown at works stage



(D) = C $\delta \sqrt{WH}$

where: C is the type of drop. Its value is given in Table. δ is a correction factor. δ = 0.9 for metastable soils, young fills, or very recent hydraulic fills and δ = 0.4 – 0.6 for sands.

Table Values of coefficient C in the equation

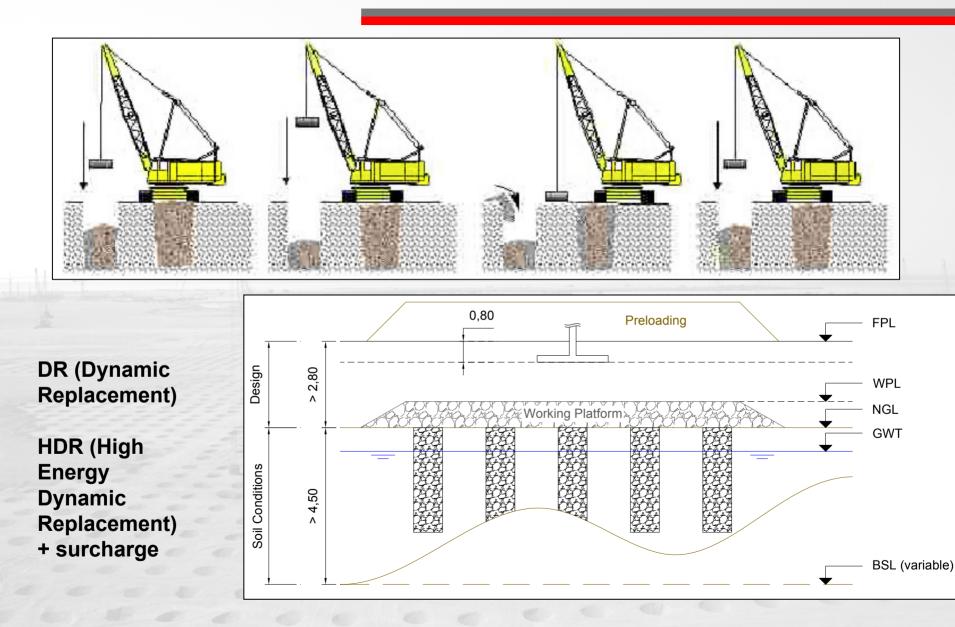
Drop method	Free drop	Rig drop	Mechanical winch	Hydraulic winch	Double hydraulic winch	
С	1.0	0.89	0.75	0.64	0.5	

The equation has been revised recently by Varaksin and Racinais (2009) as:

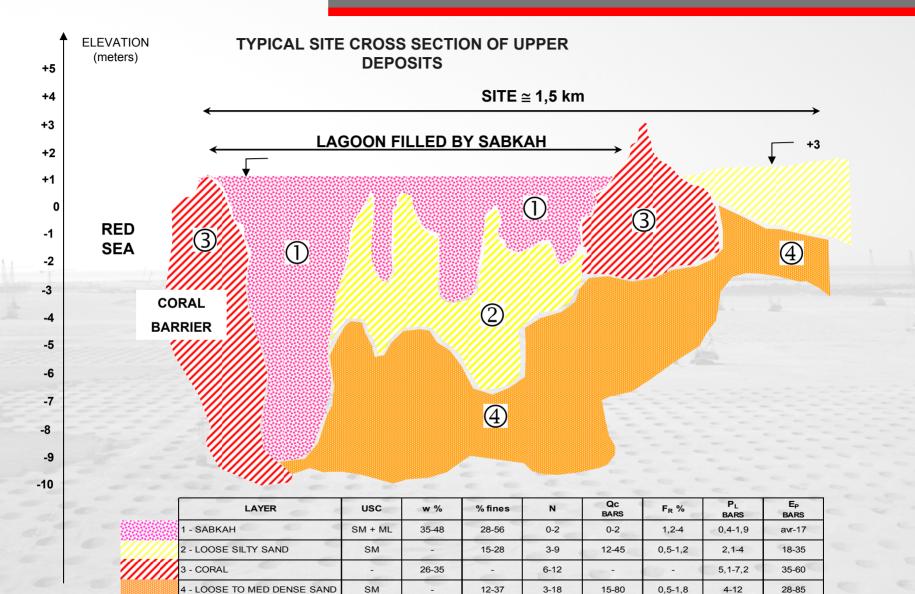
$$f(z) = \frac{f_2 - f_1}{D^2} (z - NGL)^2 + f_1$$

Where: f(z) is the improvement ratio at elevation (z); z is the depth in meters; NGL is the natural ground level; D is the depth of influence of dynamic consolidation; f_1 is the maximum improvement ratio observed at ground surface and it is dimensionless. The value may be taken as $f_1 = 0.008E$ and E is the energy in tons-meter/m²; and f_2 is the improvement ratio at the maximum depth of influence that can be achieved.

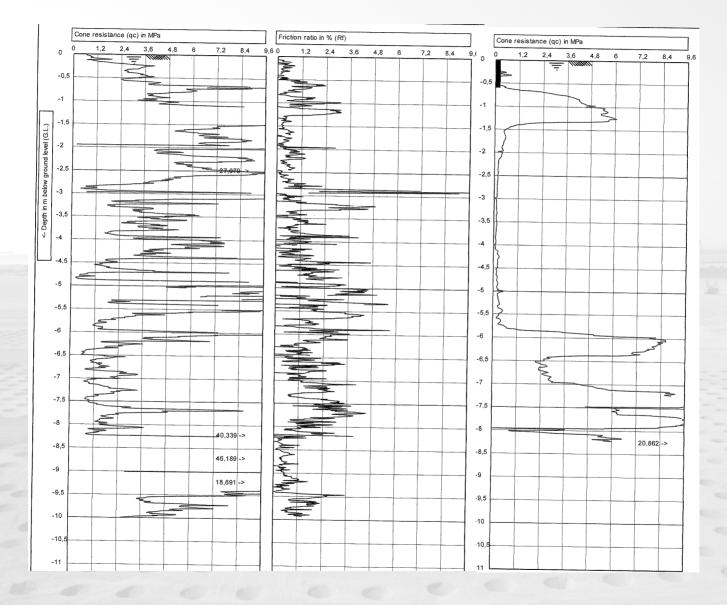
Selection of technique



Specifications



Variation in soil profile over 30 meters



Typical surface conditions

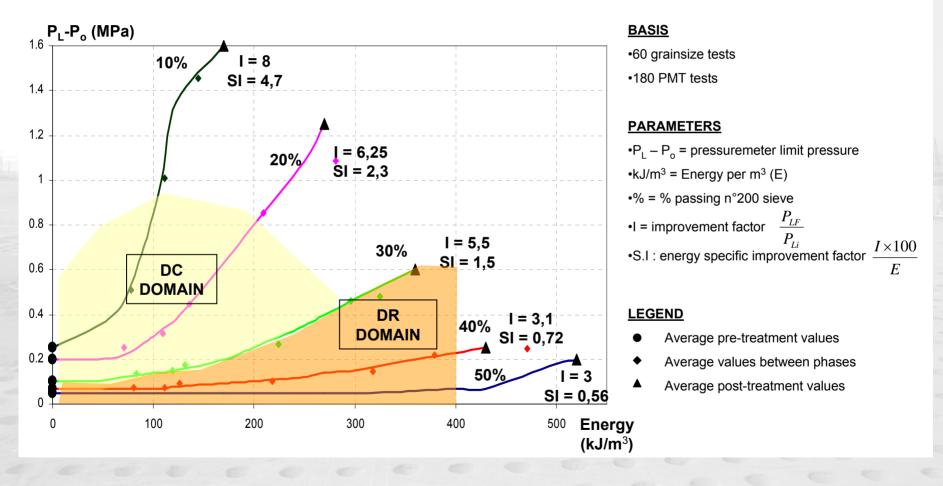




Analysis of improvement

ANALYSIS OF (P_L-P_o) IMPROVEMENT AS FUNCTION OF ENERGY AND FINES

K.A.U.S.T. – Saudi Arabia



It can be assumed that those impacts du generate a pore pressure at least equal to the pore pressure generated by the embankement load.

This new consolidation process with the final at a time $t_{\rm f}^{\rm \prime}$, where

$$T_{v} = 0,848 = \frac{C'_{v}(t'_{1}-t_{1})}{H^{2}} + \frac{C_{v}T_{1}}{H^{2}}$$

With

$$C'_{v} = C_{v} \left[1 + \frac{du}{\Delta \sigma (1 - U_{1})} \right]$$

The following equation allows to compare the respective times of consolidation being : t'_{f} with impact t. without impact

$$t'f = \frac{du}{du + \Delta\sigma(1 - U_1)}t_1 + \frac{\Delta\sigma(1 - U_1)}{du + \Delta\sigma(1 - U_1)}t_f$$

For the considered case,

du = $U\Delta\sigma$ and thus $t'_f = U_1t_1 + (1-U_1)t_f$

The Table allows to compare the gain in consolidation time, at different degrees of consolidation.

Ų	10%	20%	30%	40%	50%	60%	70%	80%	90%
t _l /t _f	0009	0.037	0.083	0.148	0231	0337	0474	0669	1.00
ť _l /t _f	0901	0.807	0.725	0.659	0615	0602	0.632	0735	1.00

Supposing primary consolidation completed

U = 0.9 or T = 0.848 if $du=U_1 \Delta \sigma$, then $t'_f = U_1 t_1 + (1-U_1) t_f$

The optimal effectiveness occurs around $U_1 = 60\%$.

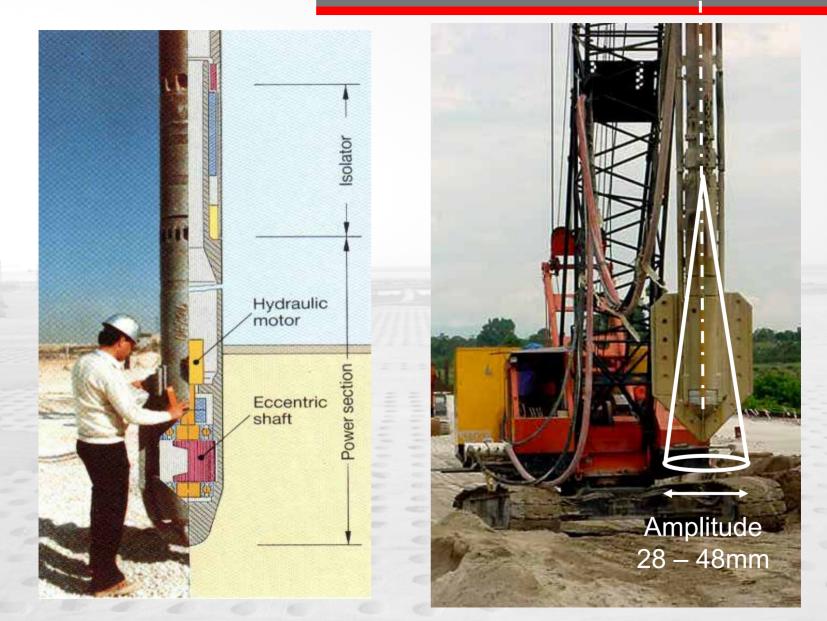
One can thus conclude that, theoretically the consolidation time is reduced by 20% to 50%, what is for practical purpose insufficient.

Dynamic surcharge





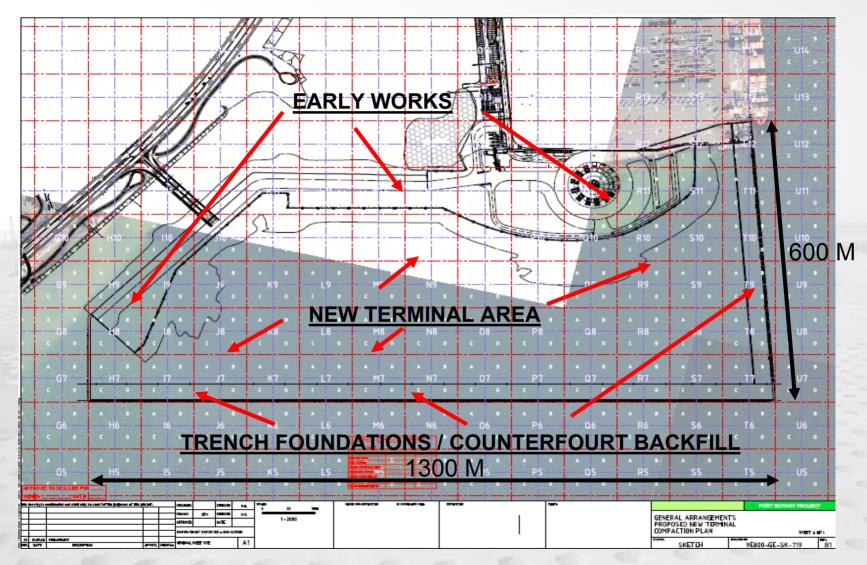
VIBROFLOTS



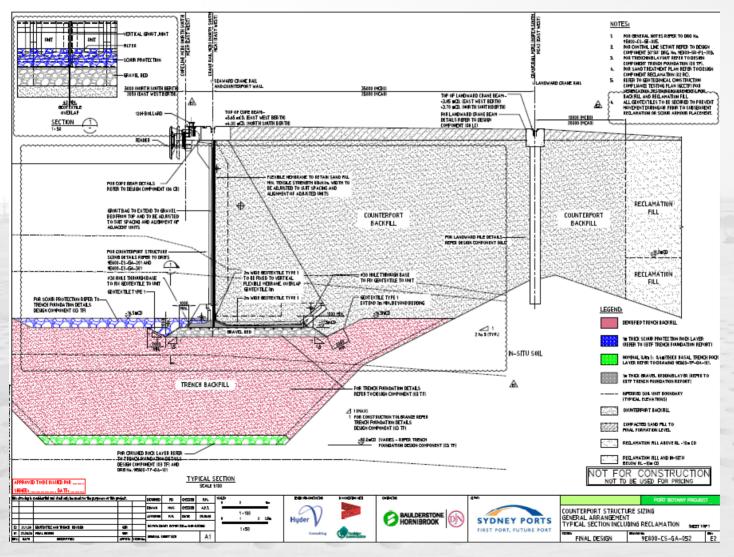
GROUND COMPACTION WORKS



GROUND COMPACTION WORKS



GENERAL ARRAGEMENT COUNTERFORTS INCLUDING RECLAMATION



RESUME / QUANTITIES

PHASE	AREA (M2)	VOLUME (M3)	TECHNIQUE
EARLY WORKS	90.000	650.000	DYNAMIC COMPACTION / VIBRO COMPACTION
TRENCH FOUNDATIONS	64,000	800.000	OFFSHORE VIBROCOMPACTION
COUNTERFOURT BACKFILL	92.000	1.330.000	ONSHORE TANDEM VIBRO COMPACTION
NEW TERMINAL AREA	404.000	5.250.000	DYNAMIC COMPACTION
TOTAL	650.000	8.000.000	DC / VC

DYNAMIC COMPACTION



POUNDER WEIGHT 25 TON / 23 METERS HIGH DROP

5.0M X 5.0M GRID / 3 PHASES - 10 BLOWS

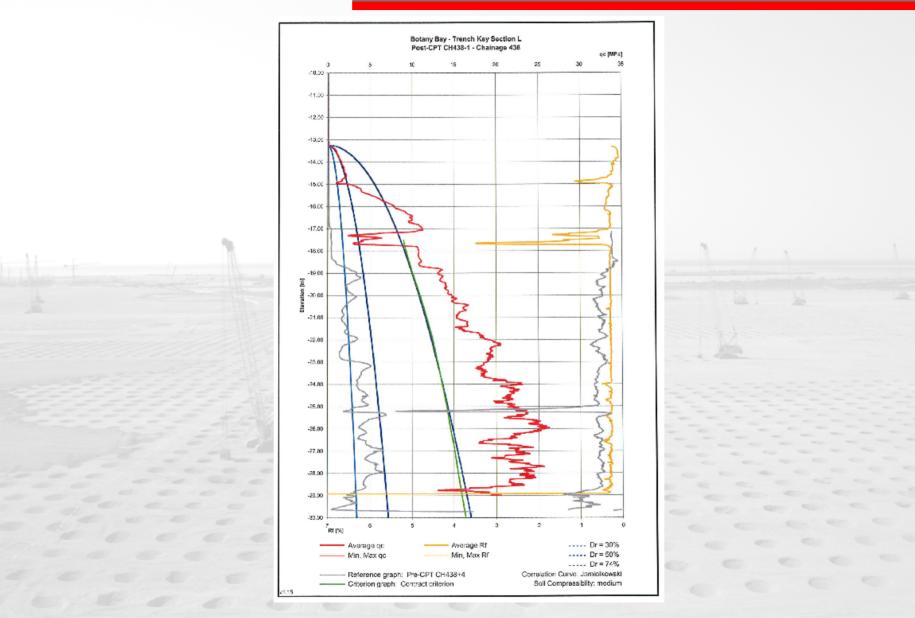
LOAD OUT WHARF – VIBRO COMPACTION V48

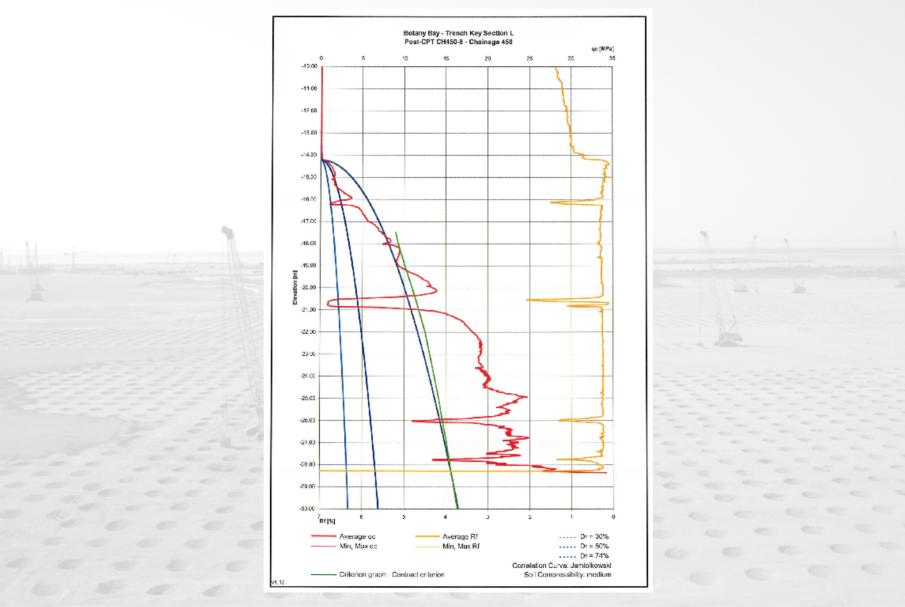


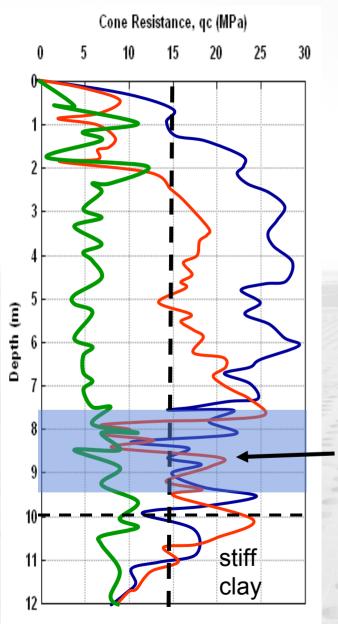
UPLIFT STEPS 1.0M / 40 SEC EACH

V48 REQUIRES WATER & AIR FOR COMPACTION

VIEW OF LOAD OUT WHARF – DC / VC WORKING







RESULTS

- 1. Except for the upper 50cm, the combination of VC and DC satisfied the q_c = 15 MPa (upper 0.5m requires surface roller compaction).
- 2. Enforced settlement: After VC – 47cm After DC – 27cm Total – 74 cm (~ 10% of treatment depth)

Compaction was less effective in this layer!

Soil Improvement Techniques

	Without added materials	With added materials
Cohesive soil	1 Drainage 2 VAcuum	<u>4 Dynamic</u> <u>replacement</u>
Peat , clay		5 Stone columns 6 CMC 7 Jet Grouting
Granular soil	3 Dynamic	8 Cement Mixing
Sand , fill	consolidation 4 Vibroflottation	

Dynamic Replacement

CONCEPT

-Very soft to stiff soils

-Unsaturated soft clays

-Thickness of less than 6 meters

-Arching layer available

PARAMETERS

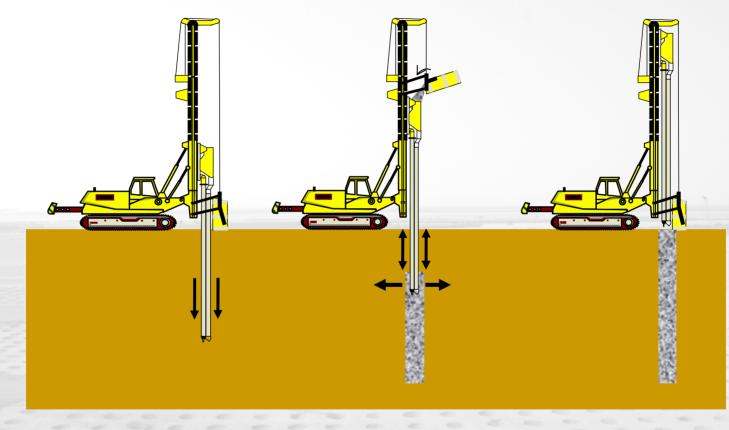
-C, \varnothing , μ , E_v of soil, column and arching

layers, grid

-or P_L , E_P , μ of soil, column and

arching layers, grid

Stone Columns – Bottom Feed



Vibrator penetration

Material feeding

Vibration of material during extraction

Principle of the technology - bottom feed with air tank

Stone Columns – Bottom Feed



Stone Columns

CONCEPT

-Soft to stiff clays

-Thickness up to 25 meters

-Arching layer available

PARAMETERS

-C, $\varnothing,\,\mu,\,E_{v}$ of soil, column and arching

layers, grid

-or P_L , E_P , μ of soil, column and arching

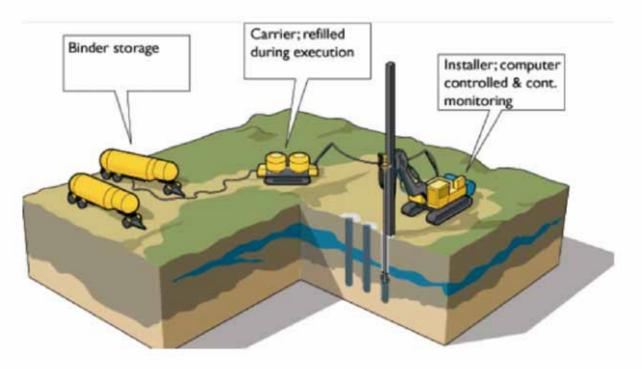
layers, grid

DCM : Deep Cement Mixing

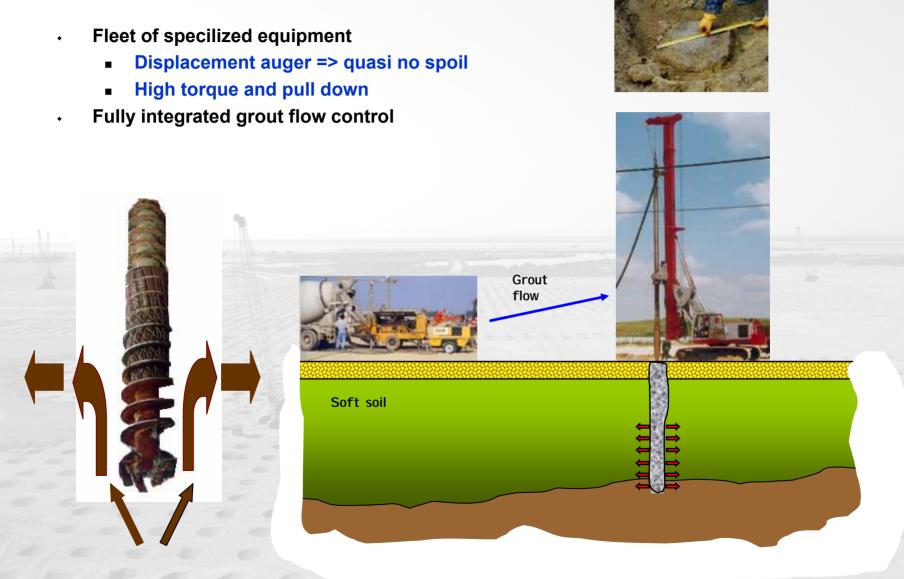
CONCEPT

The MDM process (1)

Site logistics



CMC – **Execution**



CMC – Typical Testing

Load testing on isolated CMC

- Checking of individual capacity,
- Checking of adequate soil parameters taken into account.

Compression tests on material

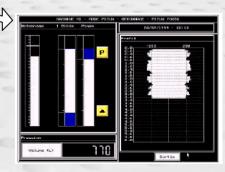
Checking of good grout resistance

Data recording system during execution

- Recording of drilling parameters => Checking of anchorage,
- Recording of grouting parameters => No necking







RIGID INCLUSIONS - PARAMETERS

SOIL

-C', Ø', E_y, μ , γ , ϕ

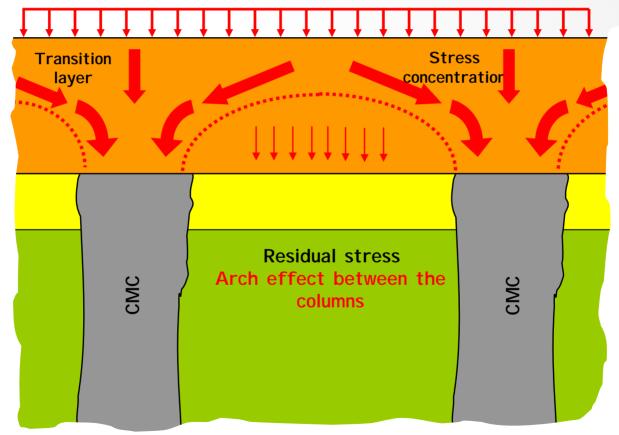
 $-K_v$, K_h if consolidation is considered

INCLUSION

-E $_{\gamma}$, μ , γ , D (non porous medium)

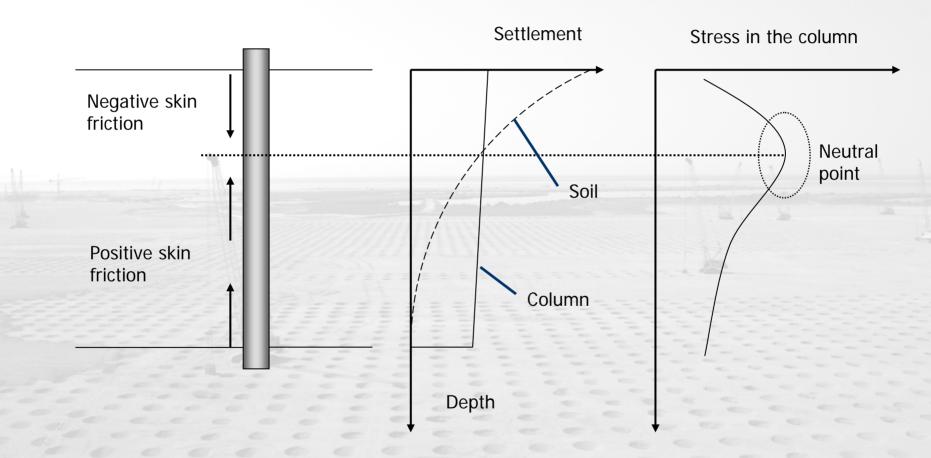
CMC Principle

- Create a <u>composite material</u> Soil + Rigid Inclusion (CMC) with:
 - Increased bearing capacity
 - Increased elastic modulus
- Transfer the load from structure to CMC network with a transition layer



CMC - Basic behavior under uniform load

Negative skin friction allows to develop a good arching effect



CMC Design - Principle

Axisymetric FEM calculation with one CMC and the soil => eq. Stiffness

Global axisymetric calculation by modelising the improved ground by material having an improved stiffness

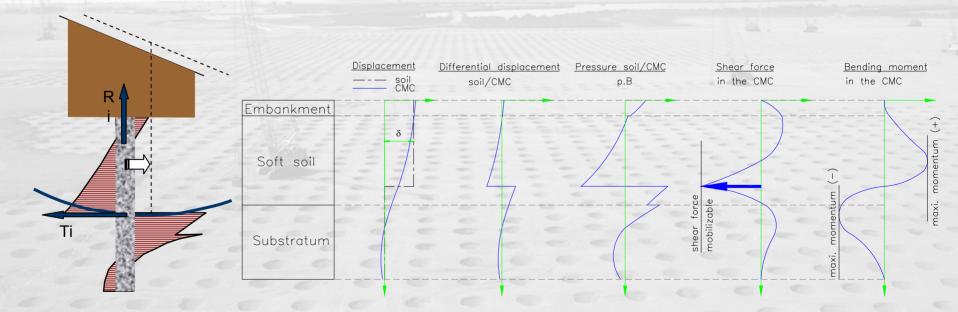
Complex Soil + CMC with improved characteristics

CMC Design – Specific case of non vertical loading

Calculation principle

1/ Estimation of the vertical stress in the column (% of the embankment load),

- 2/ Thus maximum momentum so that M / N \leq D / 8 (no traction in the mortar),
- 3/ Thus maximum shear force taken by the includion (similar to a pile to which a displacement is applied),
- 4/ Modeling of the CMC as nails working in compression + imposed shear force under TALREN software (or equivalent).

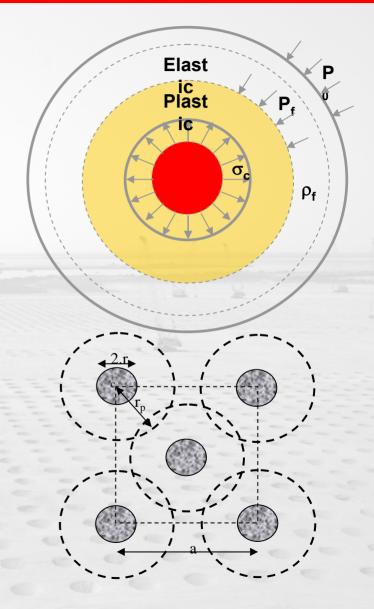


CMC Design – Benefits for the structure

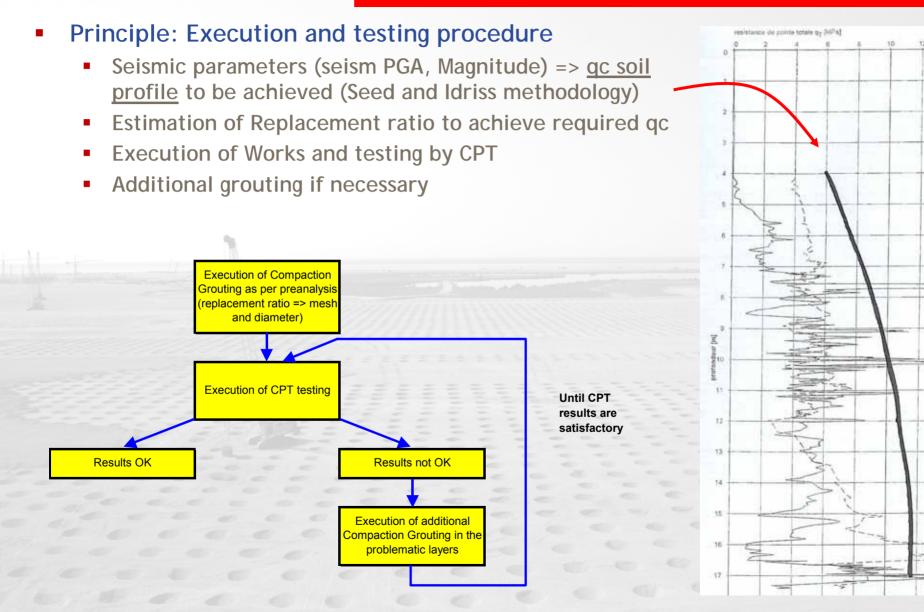
- Structure shall be designed as if soil was of good quality
 - Specialist contractor provides structural designer with bearing capacity, k, etc...
 - No connection between foundation and structure
 - Structure is less complex to be designed,
 - No stiff connection, thus no increase under seismic analysis,
 - Structure very simple to be built: footings and slab on grade, no pile cap, thus benefit in terms of cost and speed of execution

New Developement - CMC Compaction - Principle

- Aim of CMC CompactionDensify granular material to decrease liquefaction potential
- Method of densification
 - Injected mortar used to displace and compact the soil around the injection point
 - Successive injection according to a regular grid induce a global compaction of the soil
 - Mesh and diameter designed so as to achieve a given replacement ratio



New Developement - CMC Compaction - Design

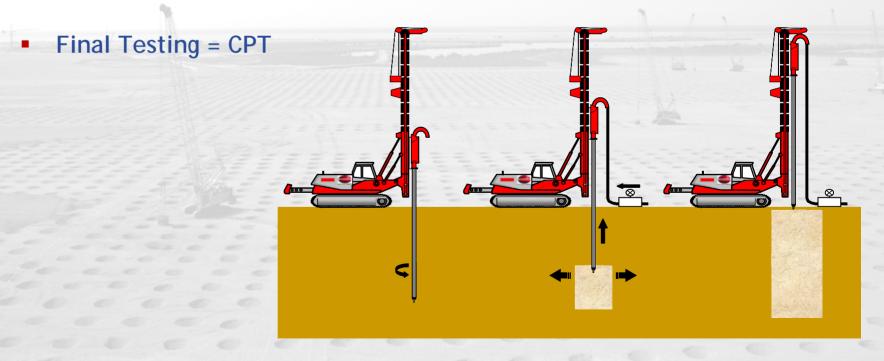


New Developement - CMC Compaction - Execution

- Same type of equipment as for CMC
 - Soil displacement rig and Pump,

Key points

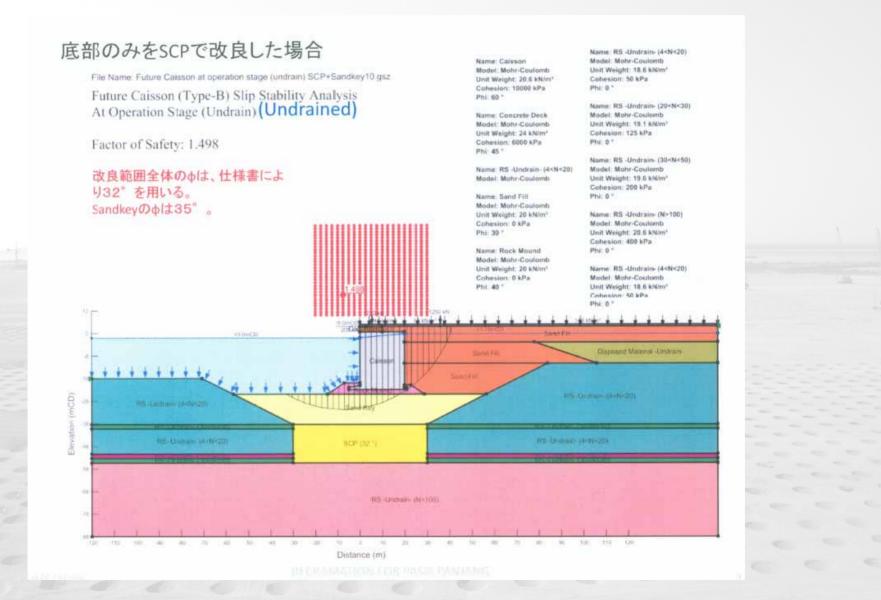
- Quality of grout (grain size distribution, workability, consistancy)
- Injection speed and successive phases



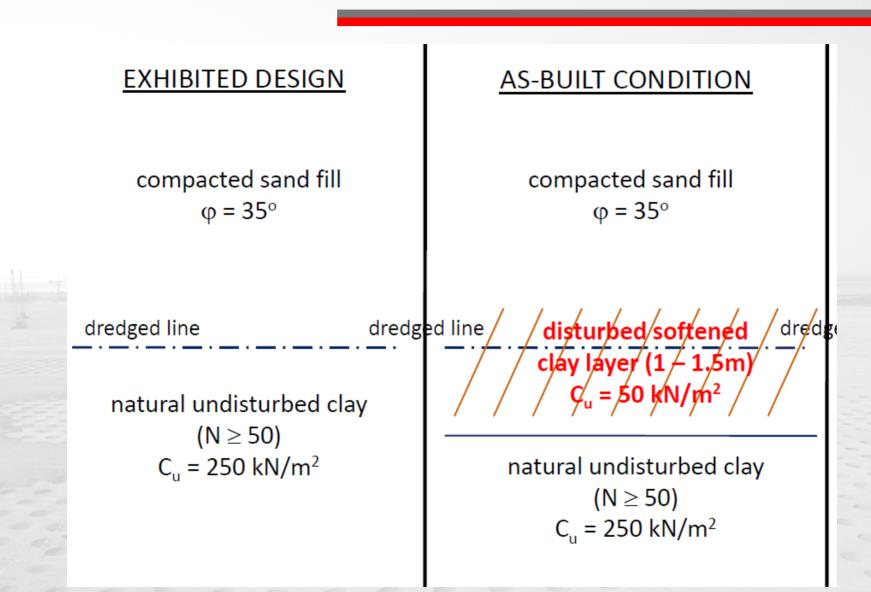
New Developement - CMC Compaction – Fos LNG Terminal



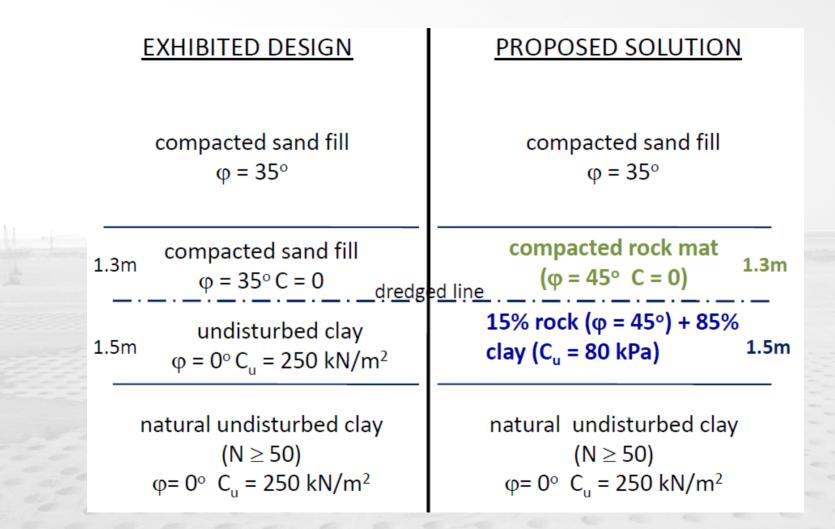
Future Caisson Stability Analysis



As built conditions



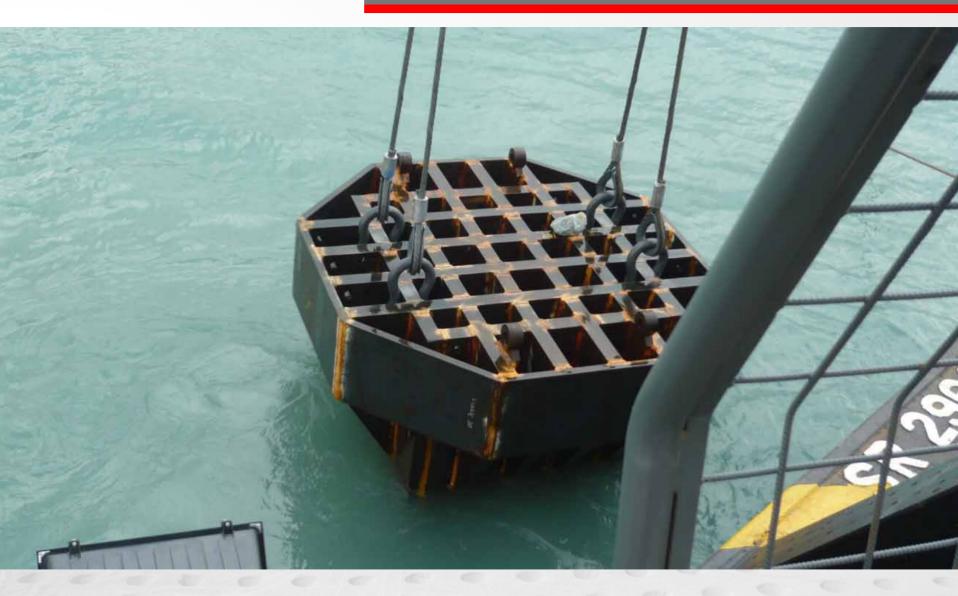
Proposed solution



View of pounder construction



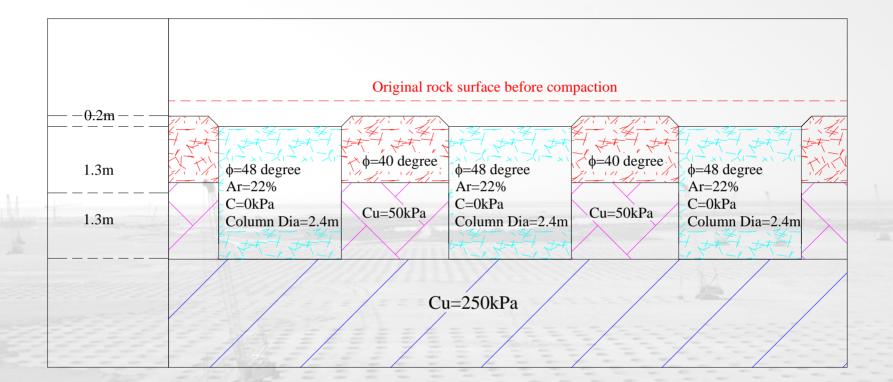
View of pounder ready to work



General SFT up



After compaction actual results





PERM – MASTER CLASS – November 16th, 2010







Derzhavin Gavriil Romanovitch



Nikolaï Semyonovitch Mordvinov