PERM – MASTER CLASS – November 16th, 2010

Concept and Parameters for Ground Improvement illustrated by case histories

by

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Chairman T.C. Ground Improvement (TC 211)

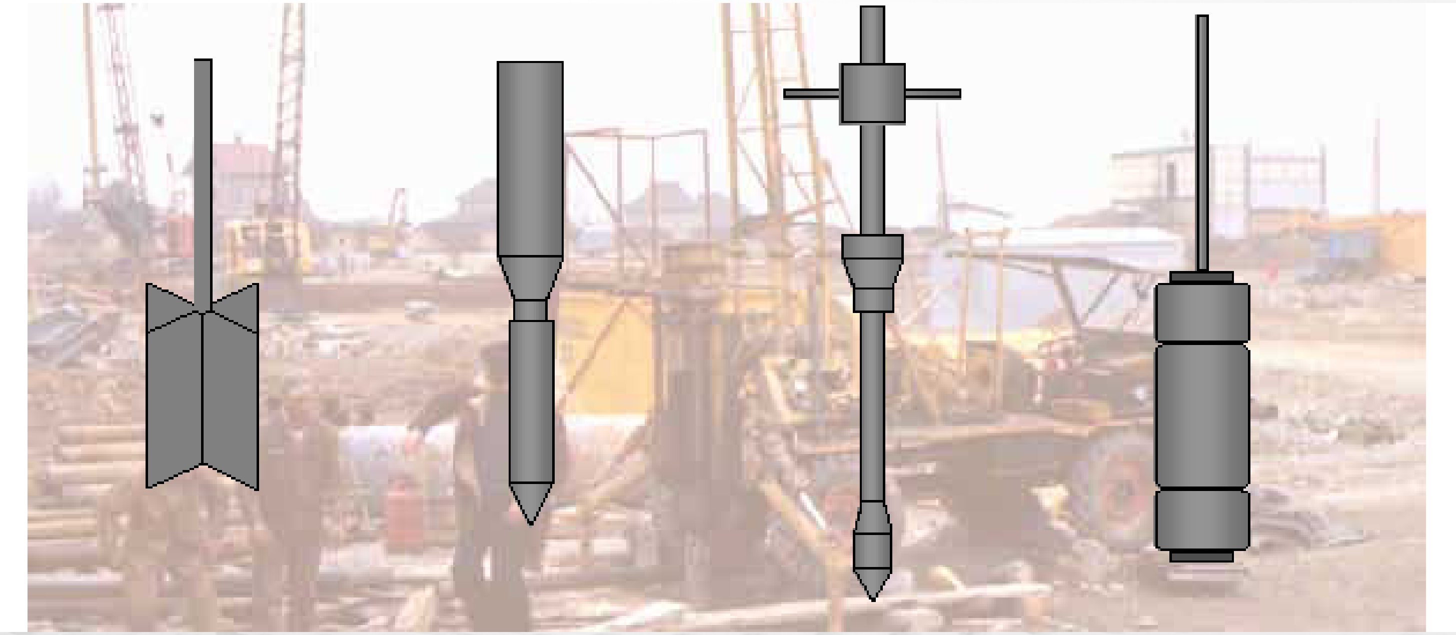
PARAMETERS RELATED TO GROUND IMPROVEMENT FROM IN SITU TESTS

PERM – MASTER CLASS – NOV 2010

Parameters related to ground improvement from in situ tests

- Menard Pressumeter (PMT)
- Static Cone Penetration (CPT)
- Dynamic Penetration (SPT)
- Vane Test (VT)
- Some correlations

Parameters related to ground improvement : Differents types of in situ tests



Vane test (VT)	Static Cone Penetration Test (CPT)	Dynamic Penetration Test (SPT)	Pressuremeter (PMT)

THE MENARD PRESSUREMETER

The Menard Pressuremeter : Typical loading tests

Typical ***load tests*** conducted on foundations :

(i) PBT; and

(ii) PMT

(*not CPT or SPT*)

PBT – vertical load test

PMT – shear loading test

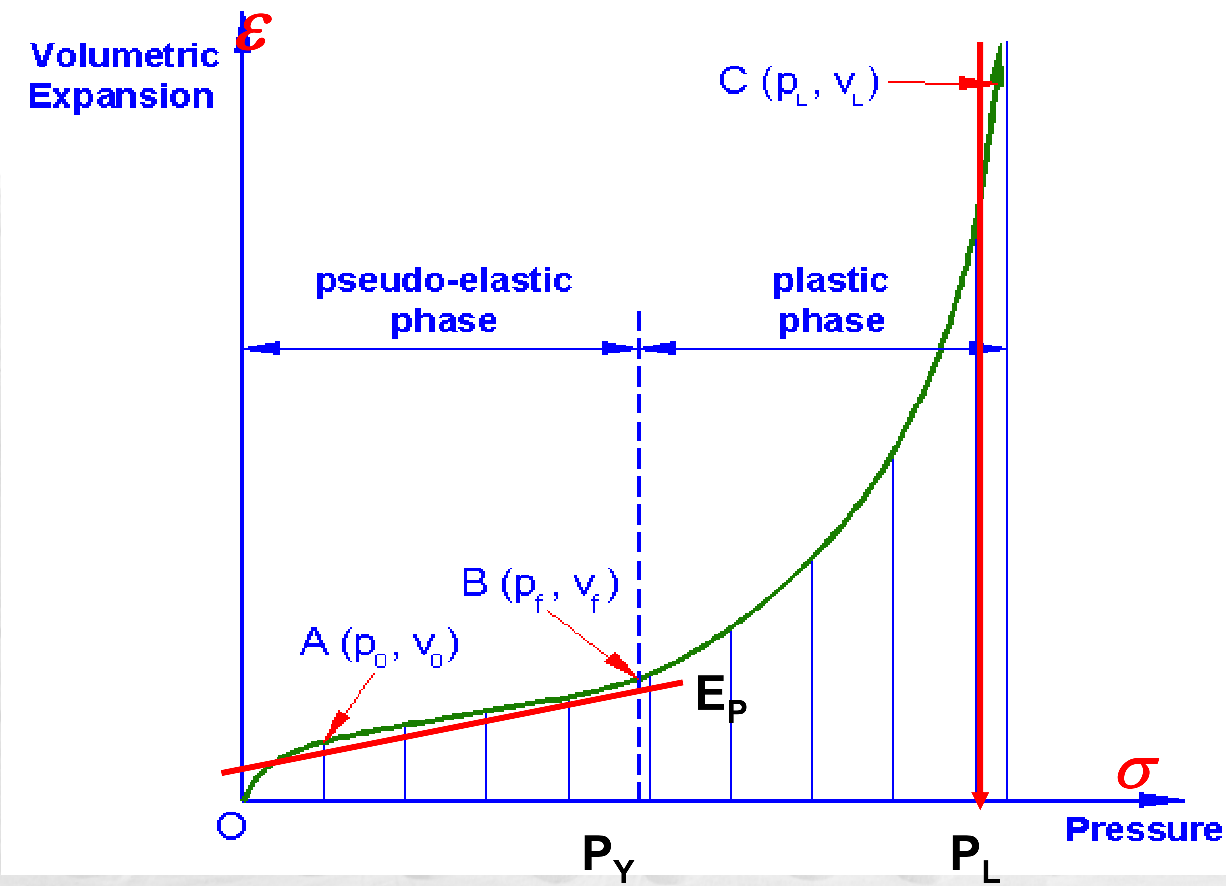
The Menard Pressuremeter : Stress – Srain curve of PMT results

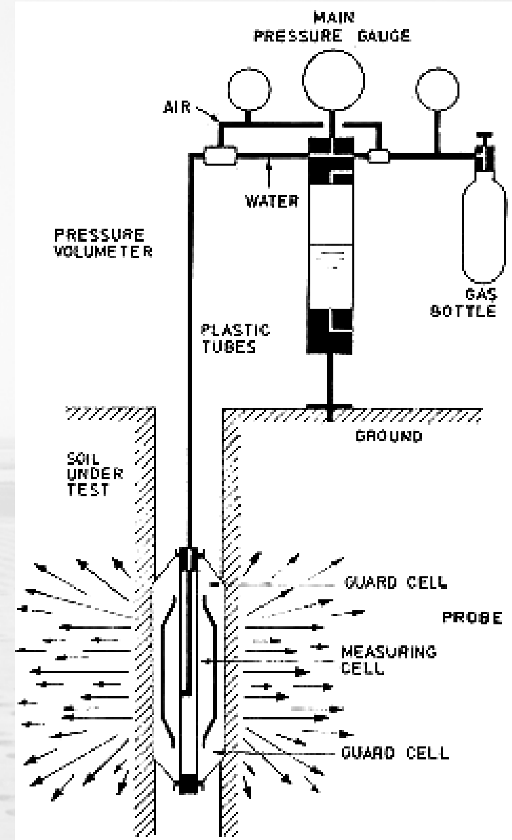
From the stress-strain (σ vs. ε) curve:

1. Limit Pressure (P_L)

– for bearing capacity (= $5.5C_u$).

2. ***Pressuremeter Modulus (E_P)*** – for settlement ($E_y = E_P/\alpha$).
(α = 2/3 for clay; 1/2 for silt and 1/3 for sand)



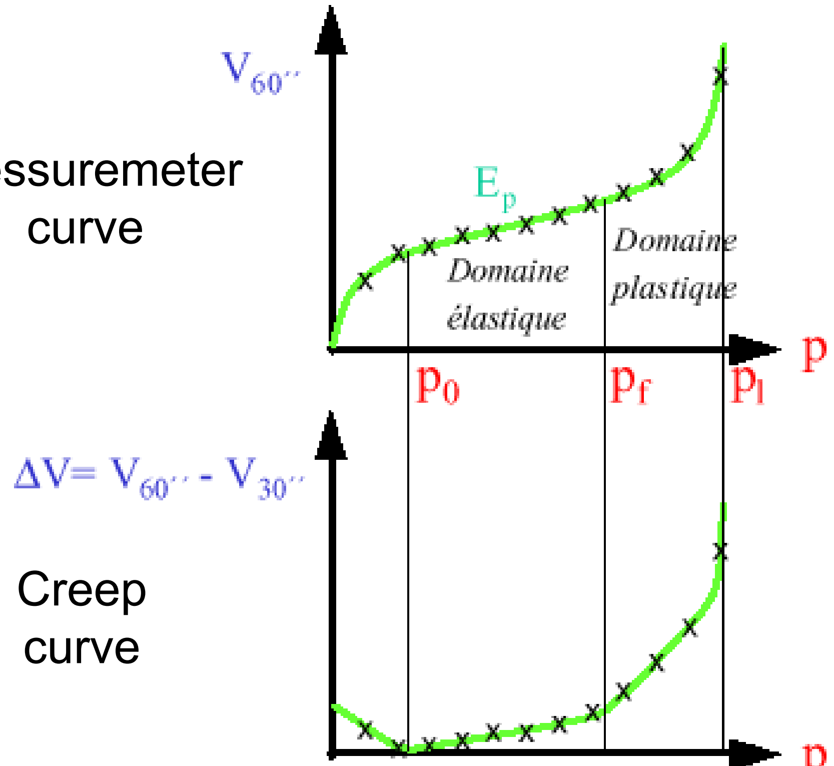


Pressure up to 40 bars acting on surrounding soil = lateral load tests.

The Menard Pressuremeter

Pressuremeter curve

Creep curve

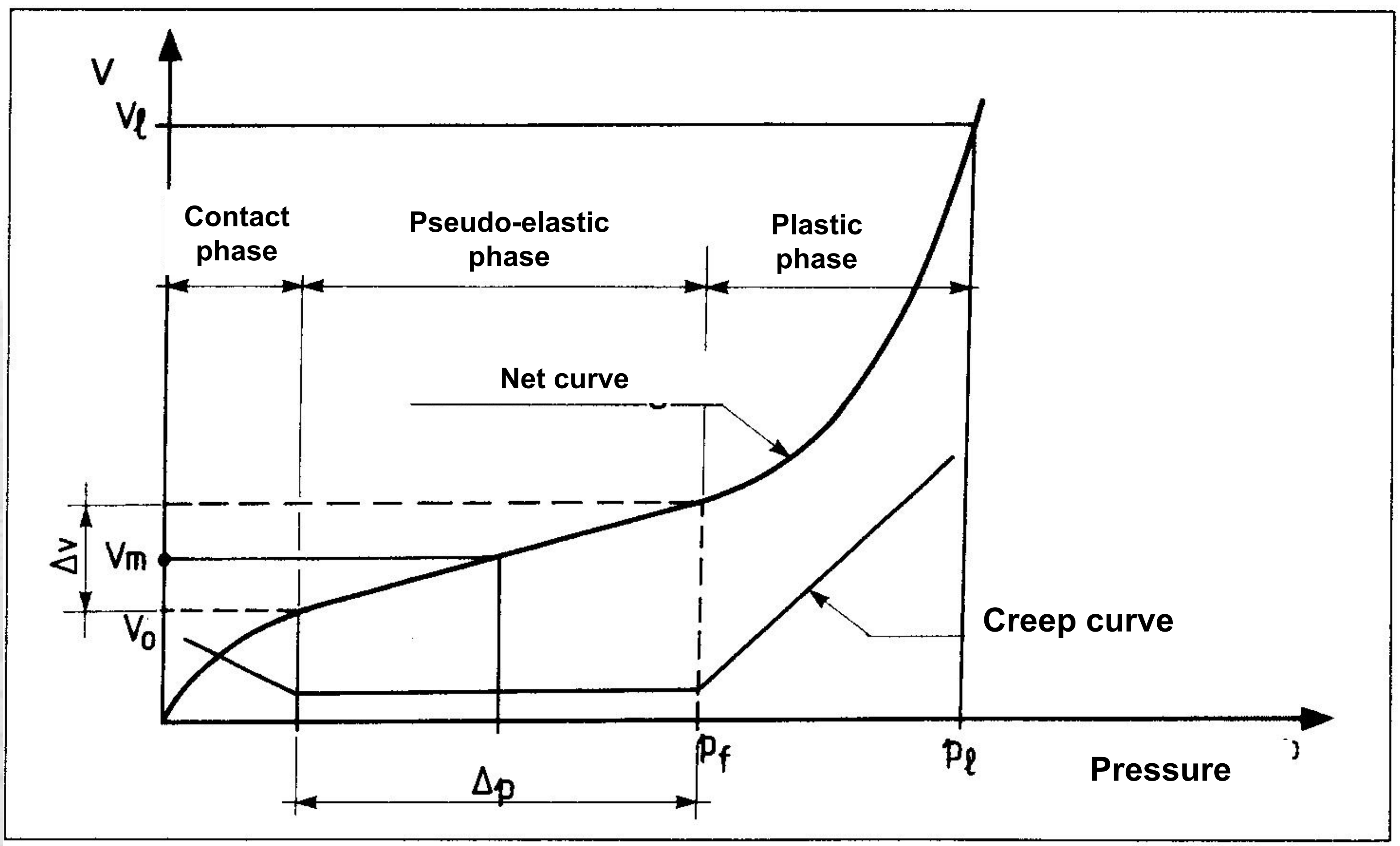


p_f Creep pressure

p_L Limit Pressure

Ep pressuremeter modulus

The Menard Pressuremeter : Net pressuremeter curve and creep curve

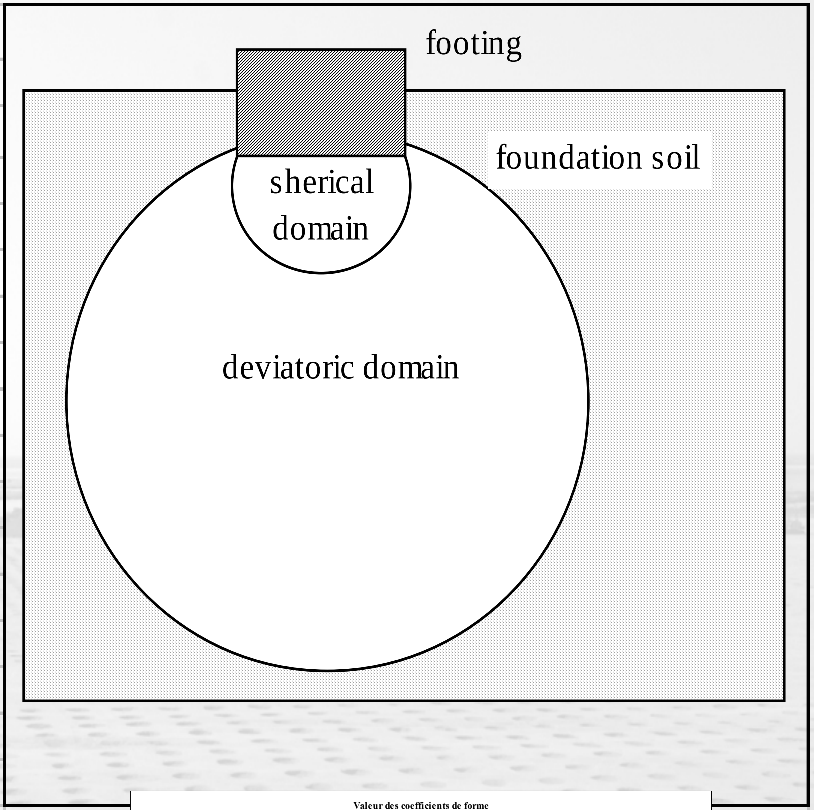


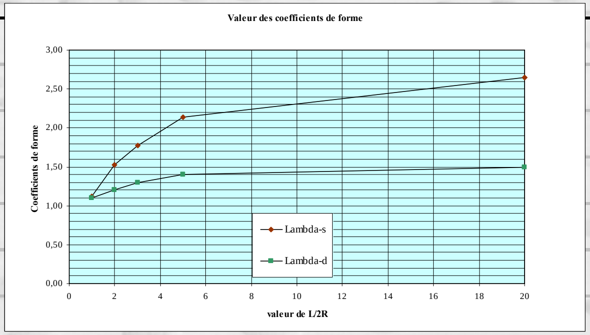
The Menard Pressuremeter : Settlement calculation under a footing

half width reference radius	R	0,30	m
Reference radius	R_0	0,30	m
Footing length	L	0,60	m
Pression sur la fondation	p	9,40	bars
PMT modulus in deviatoric domain	Ed	250	bars
PMT modulus in spherical domain	Es	250	bars
Rheological factor	α	1/4	
Shape factor	λd	1,10	
Shape factor	λs	1,12	

$$W = \frac{1.33}{3E_d} p R_0 \left(\lambda_d \frac{R}{R_0}\right)^{\alpha} + \frac{\alpha}{4.5E_s} p \lambda_s R$$

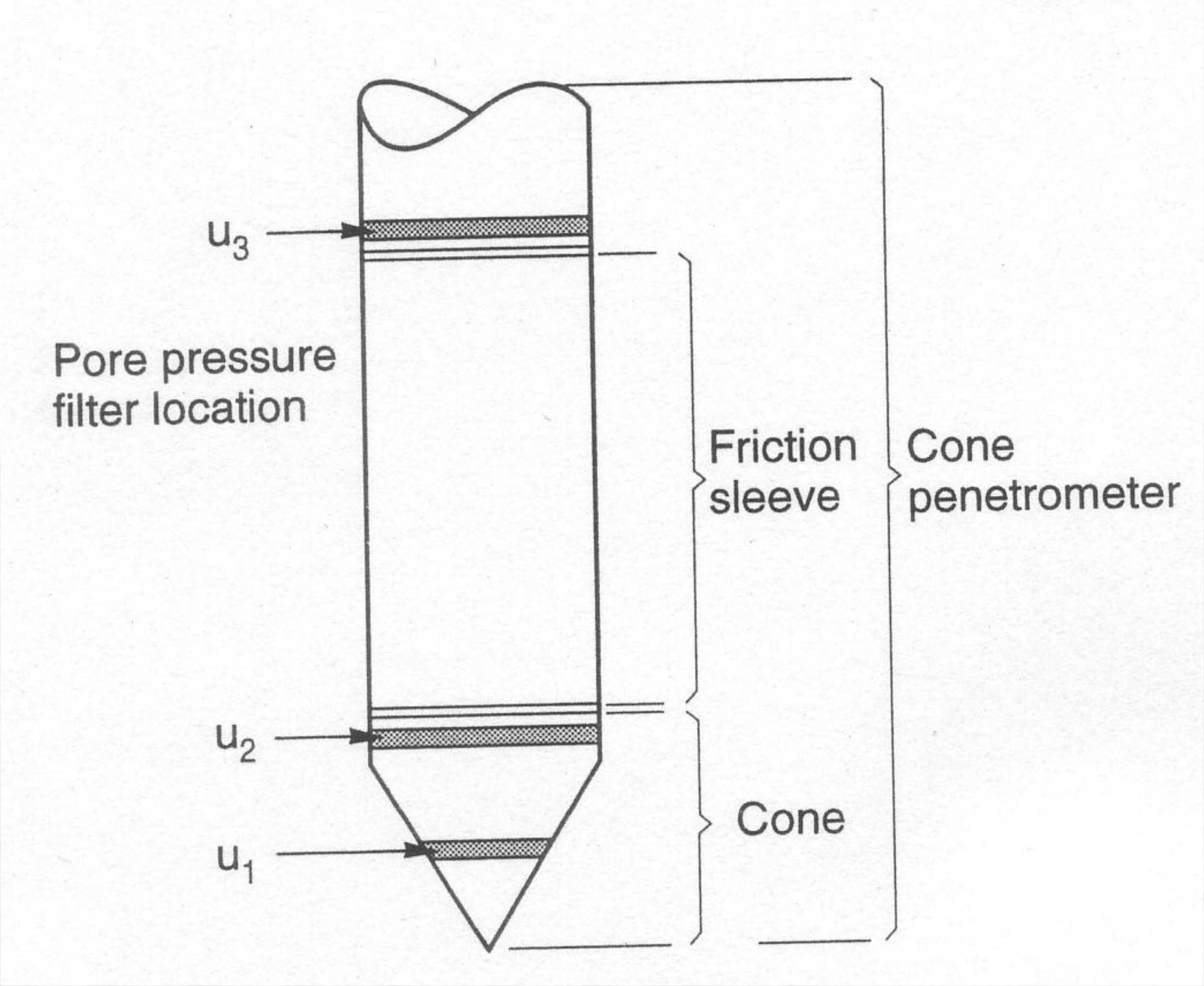
Results			
Spherical strain	Ws	0,07	cm
Deviatoric strain	Wd	0,51	cm
Calculated settlement	**W**	**0,58**	**cm**



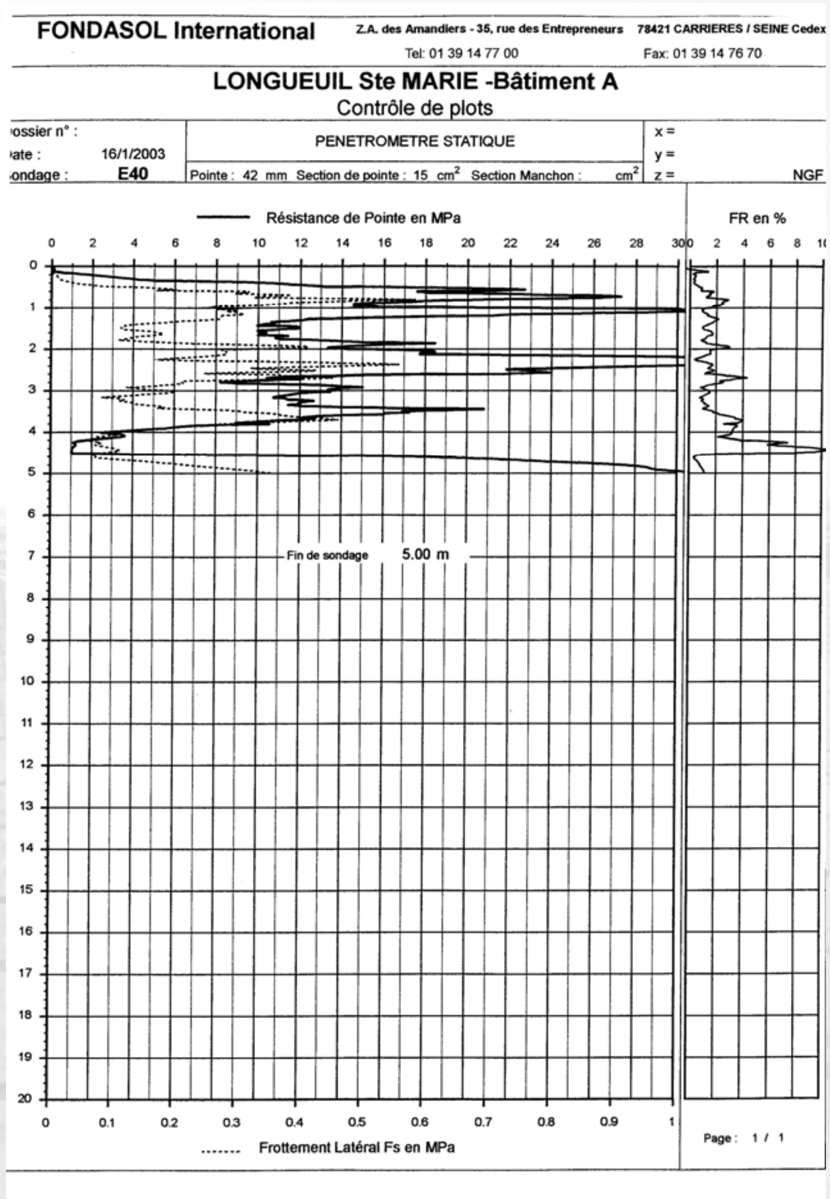


STATIC PENETRATION TEST (C.P.T.)

Static Penetration Test (C.P.T.)



Static Penetration Test : Typical CPT Test



Static Penetration Test : Rought soil identification from CPT Test

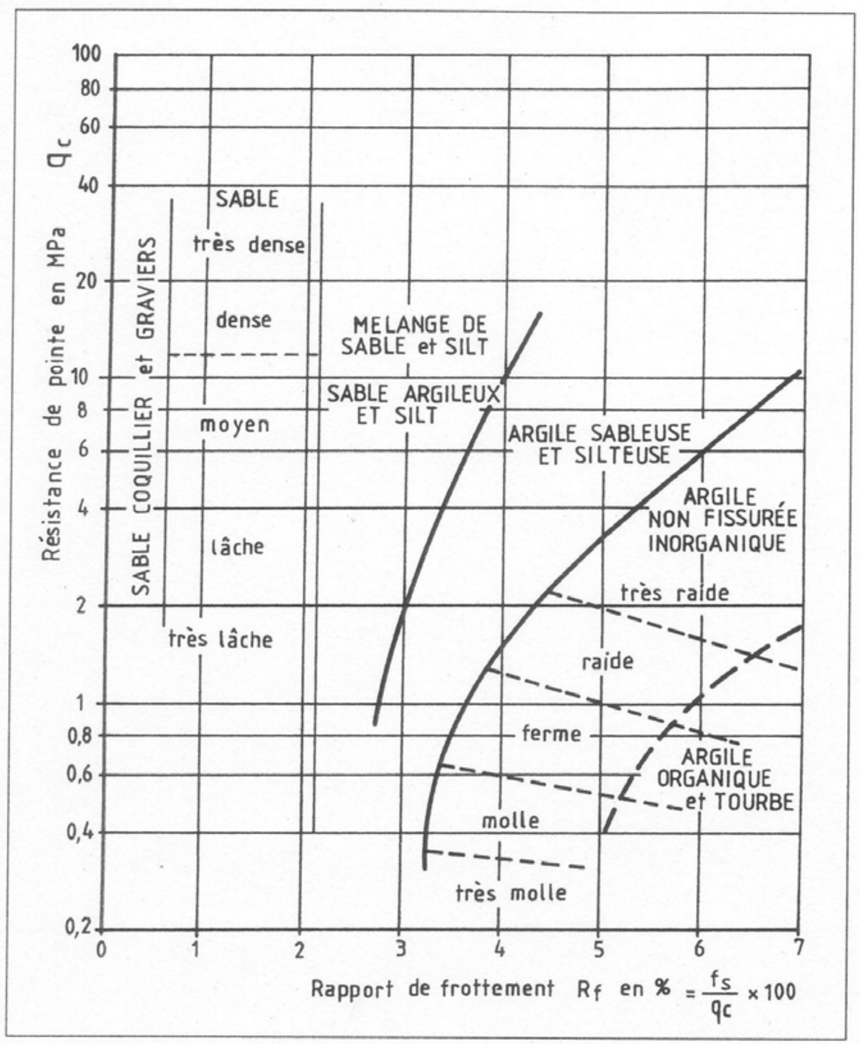
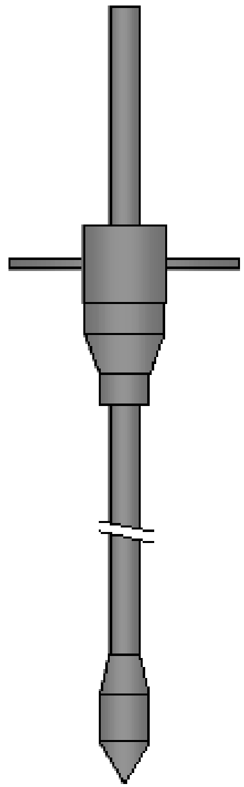


Fig. 6.14. Estimation de la nature des sols d'après q_c et R_f
(Schmertmann 1969)

DYNAMIC PENETRATION TEST (SPT, DPT)

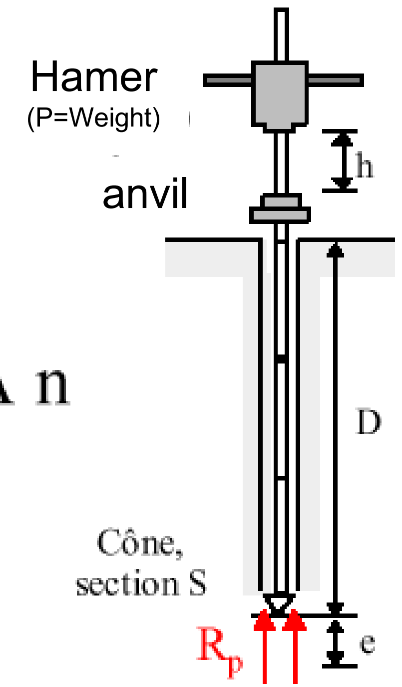
Dynamic Penetration Test



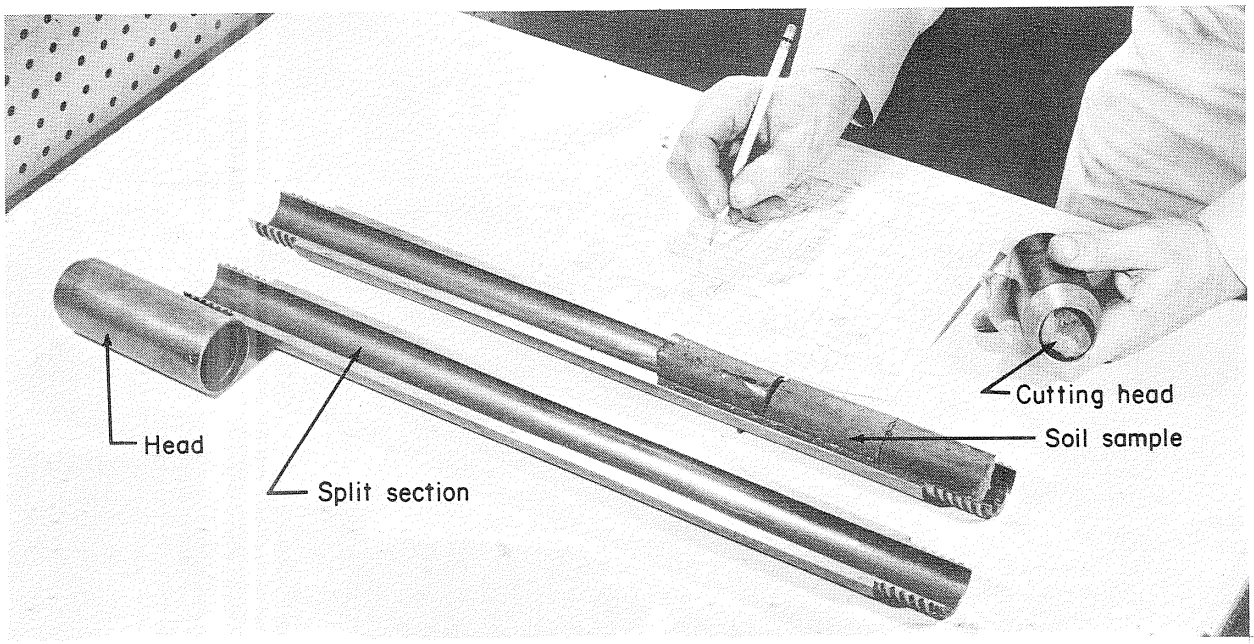
Von Moos DPT

Driving conical pond

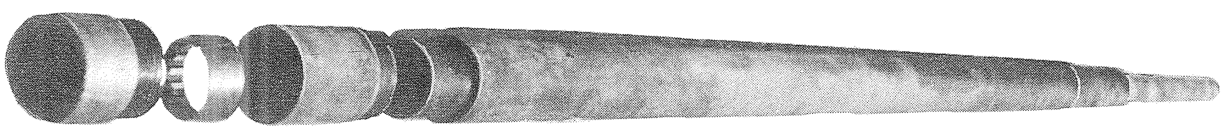
$$R_p = \frac{n}{e} \; \frac{P\,h}{S} = A\,n$$



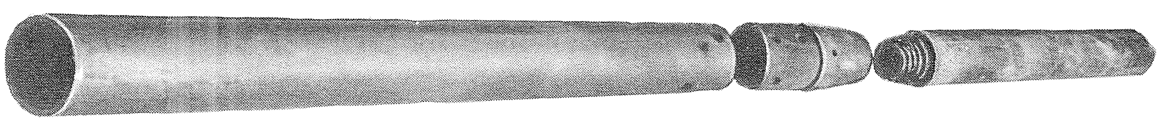
Dynamic Penetration Test



(d)



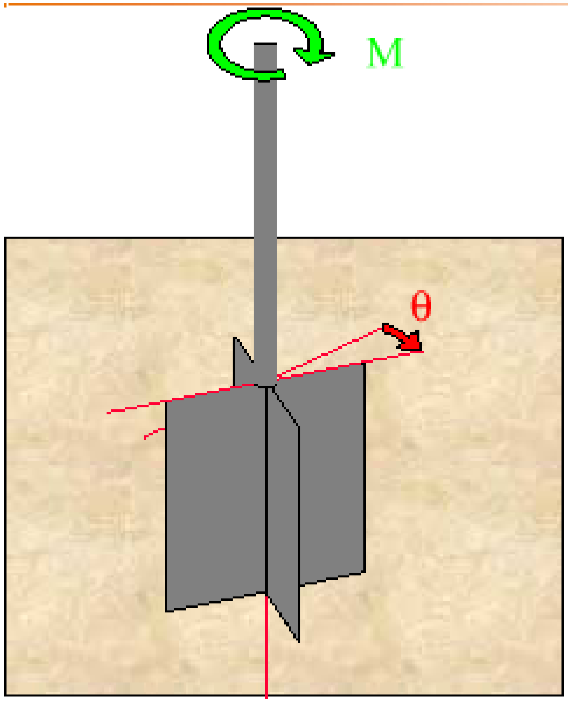
(e)



(f)

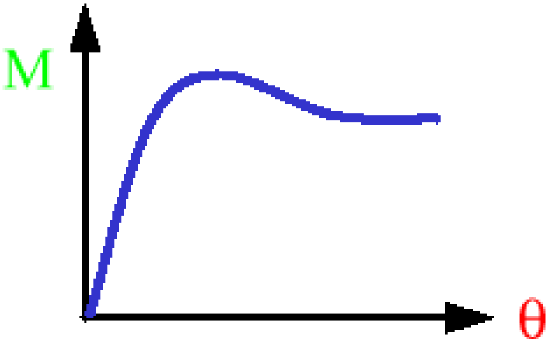
VANE TEST
(only in soft homogeneous clay)

Vane Test



Cohesive soil

$$C_u = \frac{6M}{\pi D^2 (3H + D)}$$



➡ Undrained cohesion of soils

State of the Art Report

17TH International Conference on
Soil Mechanics & Geotechnical Engineering

State of the Art Report

Construction Processes
Procédés de Construction

Jian Chu
Nanyang Technological University, Singapore
Serge Varaksin
Menard, France
Ulrich Klotz
Zublin International GmbH, Germany
Patrick Mengé
Dredging International n.v., DEME, Belgium

NOTA : TC 17 meeting ground improvement – 07/10/2009
Website : www.bbri.be/go/tc17

Category	Method	Principle
A. Ground improvement without admixtures in non-cohesive soils or fill materials	A1. Dynamic compaction	Densification of granular soil by dropping a heavy weight from air onto ground.
	A2. Vibrocompaction	Densification of granular soil using a vibratory probe inserted into ground.
	A3. Explosive compaction	Shock waves and vibrations are generated by blasting to cause granular soil ground to settle through liquefaction or compaction.
	A4. Electric pulse compaction	Densification of granular soil using the shock waves and energy generated by electric pulse under ultra-high voltage.
	A5. Surface compaction (including rapid impact compaction).	Compaction of fill or ground at the surface or shallow depth using a variety of compaction machines.
B. Ground improvement without admixtures in cohesive soils	B1. Replacement/displacement (including load reduction using light weight materials)	Remove bad soil by excavation or displacement and replace it by good soil or rocks. Some light weight materials may be used as backfill to reduce the load or earth pressure.
	B2. Preloading using fill (including the use of vertical drains)	Fill is applied and removed to pre-consolidate compressible soil so that its compressibility will be much reduced when future loads are applied.
	B3. Preloading using vacuum (including combined fill and vacuum)	Vacuum pressure of up to 90 kPa is used to pre-consolidate compressible soil so that its compressibility will be much reduced when future loads are applied.
	B4. Dynamic consolidation with enhanced drainage (including the use of vacuum)	Similar to dynamic compaction except vertical or horizontal drains (or together with vacuum) are used to dissipate pore pressures generated in soil during compaction.
	B5. Electro-osmosis or electro-kinetic consolidation	DC current causes water in soil or solutions to flow from anodes to cathodes which are installed in soil.
	B6. Thermal stabilisation using heating or freezing	Change the physical or mechanical properties of soil permanently or temporarily by heating or freezing the soil.
	B7. Hydro-blasting compaction	Collapsible soil (loess) is compacted by a combined wetting and deep explosion action along a borehole.

C. Ground improvement with admixtures or inclusions	C1. Vibro replacement or stone columns	Hole jetted into soft, fine-grained soil and back filled with densely compacted gravel or sand to form columns.
	C2. Dynamic replacement	Aggregates are driven into soil by high energy dynamic impact to form columns. The backfill can be either sand, gravel, stones or demolition debris.
	C3. Sand compaction piles	Sand is fed into ground through a casing pipe and compacted by either vibration, dynamic impact, or static excitation to form columns.
	C4. Geotextile confined columns	Sand is fed into a closed bottom geotextile lined cylindrical hole to form a column.
	C5. Rigid inclusions (or composite foundation, also see Table 5)	Use of piles, rigid or semi-rigid bodies or columns which are either premade or formed in-situ to strengthen soft ground.
	C6. Geosynthetic reinforced column or pile supported embankment	Use of piles, rigid or semi-rigid columns/inclusions and geosynthetic girds to enhance the stability and reduce the settlement of embankments.
	C7. Microbial methods	Use of microbial materials to modify soil to increase its strength or reduce its permeability.
	C8 Other methods	Unconventional methods, such as formation of sand piles using blasting and the use of bamboo, timber and other natural products.

D. Ground improvement with grouting type admixtures	D2. Chemical grouting	Solutions of two or more chemicals react in soil pores to form a gel or a solid precipitate to either increase the strength or reduce the permeability of soil or ground.
	D3. Mixing methods (including premixing or deep mixing)	Treat the weak soil by mixing it with cement, lime, or other binders in-situ using a mixing machine or before placement
	D4. Jet grouting	High speed jets at depth erode the soil and inject grout to form columns or panels
	D5. Compaction grouting	Very stiff, mortar-like grout is injected into discrete soil zones and remains in a homogenous mass so as to densify loose soil or lift settled ground.
	D6. Compensation grouting	Medium to high viscosity particulate suspensions is injected into the ground between a subsurface excavation and a structure in order to negate or reduce settlement of the structure due to ongoing excavation.
E. Earth reinforcement	E1. Geosynthetics or mechanically stabilised earth (MSE)	Use of the tensile strength of various steel or geosynthetic materials to enhance the shear strength of soil and stability of roads, foundations, embankments, slopes, or retaining walls.
	E2. Ground anchors or soil nails	Use of the tensile strength of embedded nails or anchors to enhance the stability of slopes or retaining walls.
	E3. Biological methods using vegetation	Use of the roots of vegetation for stability of slopes.

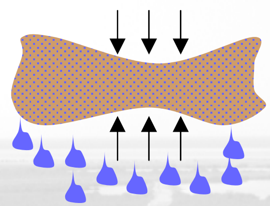
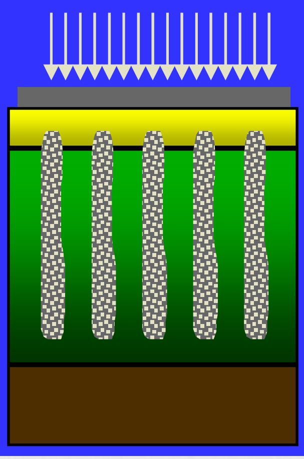
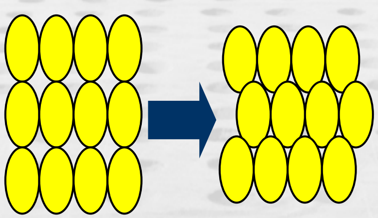
Why Soil improvement ?

- To increase bearing capacity and stability (avoid failure)
- To reduce post construction settlements
- To reduce liquefaction risk (sismic area)

Advantages / classical solutions

- avoid deep foundation (price reduction also on structure work like slab on pile)
- avoid soil replacement
- save time
- Avoid to change site
- Save money !

Soil Improvement Techniques

	Without added materials	With added materials
Cohesive soil **Peat , clay …**	1 Drainage 2 VAcuum 	4 Dynamic replacement 5 Stone columns 6 CMC 7 Jet Grouting 8 Cement Mixing 
Soil with friction **Sand , fill**	3 Dynamic consolidation 4 Vibroflottation 	

Parameters For Concept

-Soil caracteristics

-cohesive or non cohesive

- blocks ?

- Water content, water table position

- Organic materials

-Soil thickness

-Structure to support

-Isolated or uniform load

-Deformability

-Site environment

-Close to existing structure

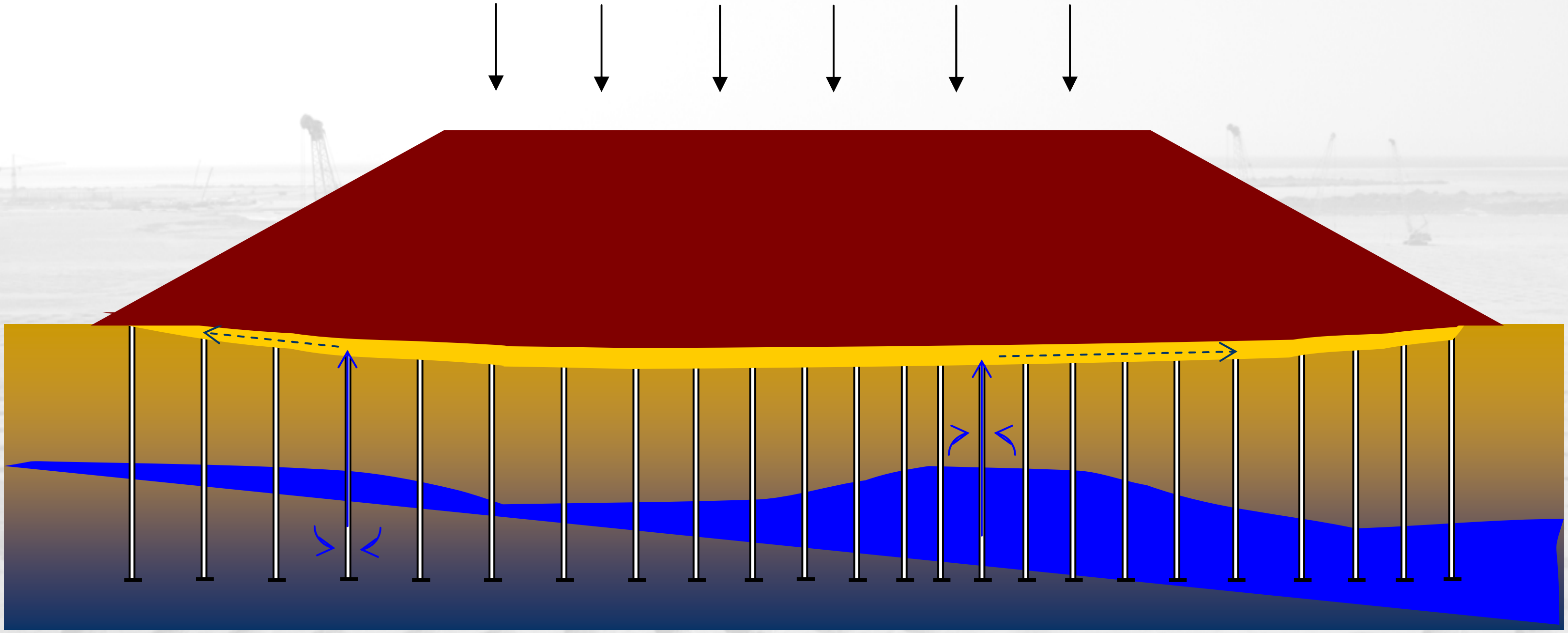
-Height constraints

-Time available to build

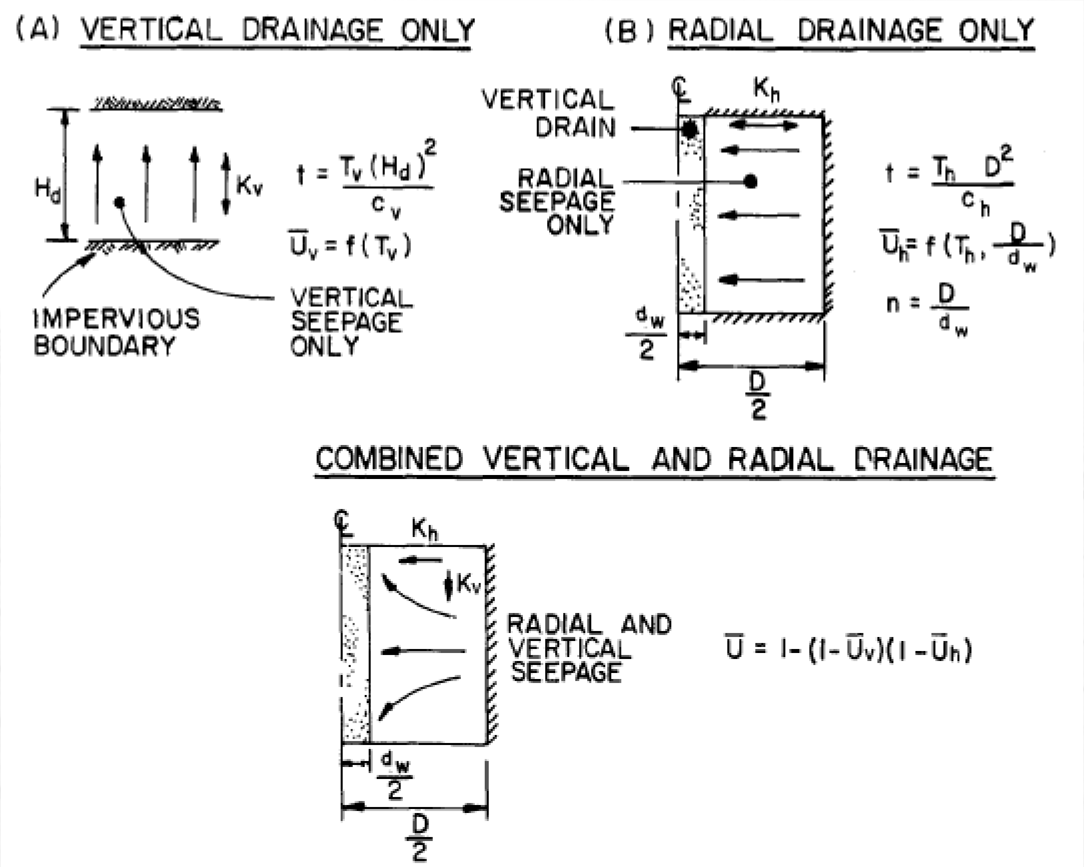
Preloading with vertical drains

high fines contents soils

$$\sigma=\sigma'+u$$



Radial and Vertical consolidation



Vertical drains: material

High fines contents soils

5 cm , PVC

Flat drain circular drain *vertical drain + geotextile*

Vertical Drains

Vertical drains

CONCEPT

-Stable subsoil for surcharge

-Soil can be penetrated

-Time available is short

-Some residual settlement is allowed

PARAMETERS

1 – Depth

2 – Drainage path

3 – Cohesion

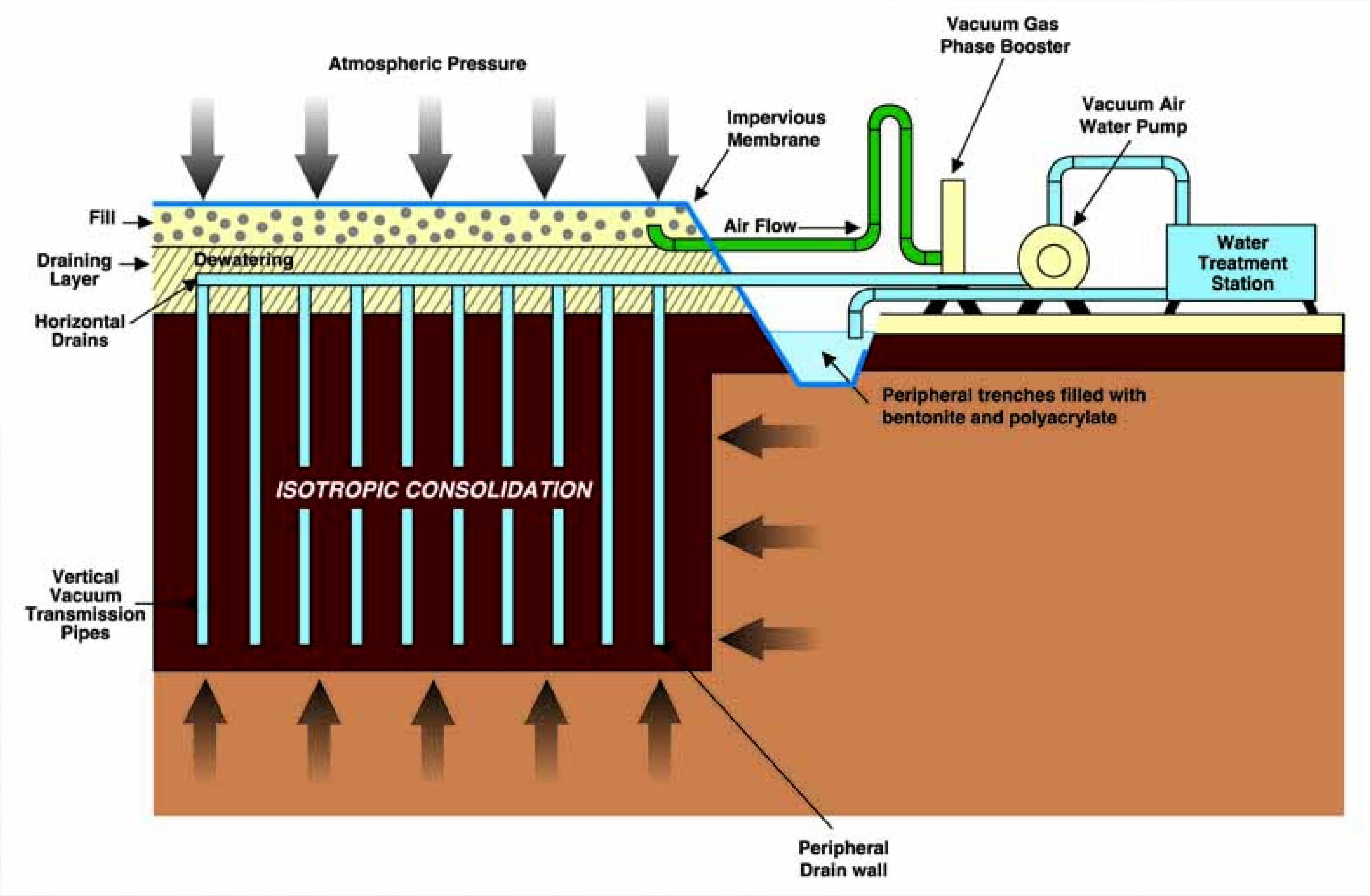
4 – Consolidation parameters

(oedometer, CPT)

e_O, C_C, C_V, C_R, C_α, t,

CPT dissipation test

Vacuum Consolidation (high fines contents soils)



VACUUM (J.M. COGNON PATENT)

Vacuum Consolidation

CONCEPT

-Soil is too soft for surcharge

-Time does not allow for step loading

-Surcharge soil not available

-Available area does not allow for berns

PARAMETERS

1 – Depth
2 – Drainage path
3 – Condition of impervious soil
4 – Watertable near surface
5 - Absence of pervious continuous layer
6 – Cohesion
7 - Consolidation parameters (oedometer, CPT)
e_O, C_C, C_V, C_R, C_α, t,
CPT dissipation test
8 – Theoretical depression value
9 – Field coefficient vacuum
10 – Reach consolidation to effective pressure in every layer
11 – Target approach

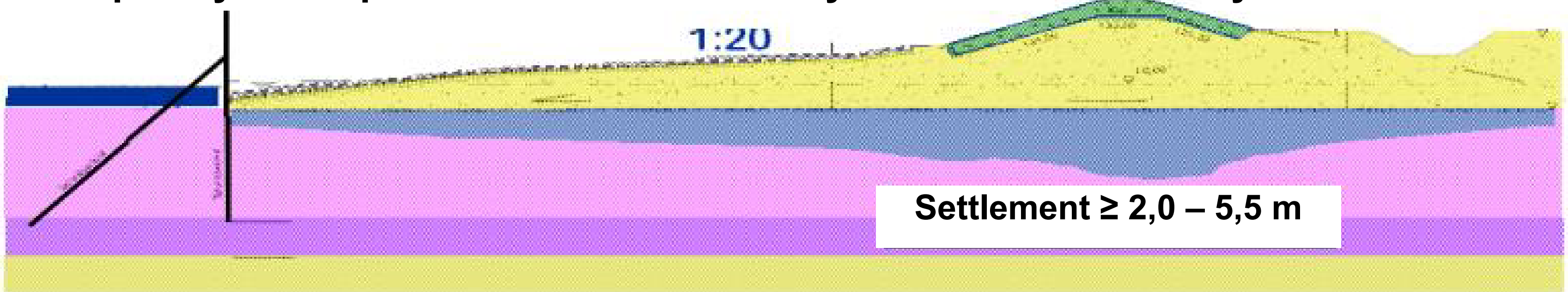

A380
A380

Case history – EADS Airbus Plant, Hamburg

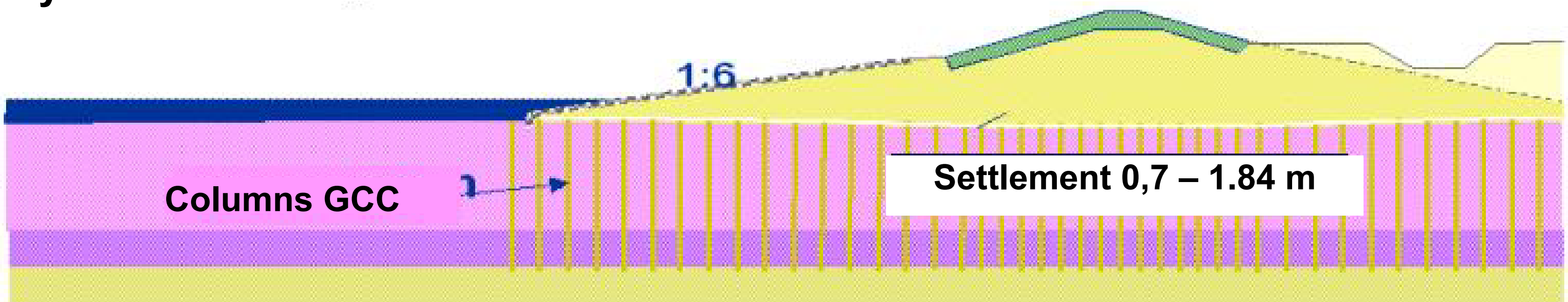
General overview of Airbus site

Basic design and alternate concept of Moebius–Menard

Temporary sheet pile wall – in 5 month – dyke construction in 3 years



Dyke construction to +6.5 in 8.5 month and to + 9.00 in 16 month



Subsoil characteristics

Soil type	Water content	Density	Shear strength		Deformation Modulus (under σ_z = 100 kN/m²)	Coefficient of consolidation	Coefficient of secondary consolidation
	W (%)	γ/γ' – kN/m³	δ'(°)/c' (kN/m²)	C_u (kN/m²)	E_S (MN/m²)	C_V (m²/year)	$C\alpha$ (-)
Mud	142	13/3	20/0	0.5-5	0.8	0.35	0.03
Young clay	119	14/4	20/0	2-10	0.9	0.35	0.03
Clay	70	15/5	17.5/10	5-20	1.5	0.5	0.02
Peaty clay	139	14/4	20/5	5-20	0.9	0.4	0.03
Peat	240	11/1	20/0	5-15	0.5	$\geq$ 0.4	0.04

Case history – EADS Airbus Plant, Hamburg

Case history – EADS Airbus Plant, Hamburg

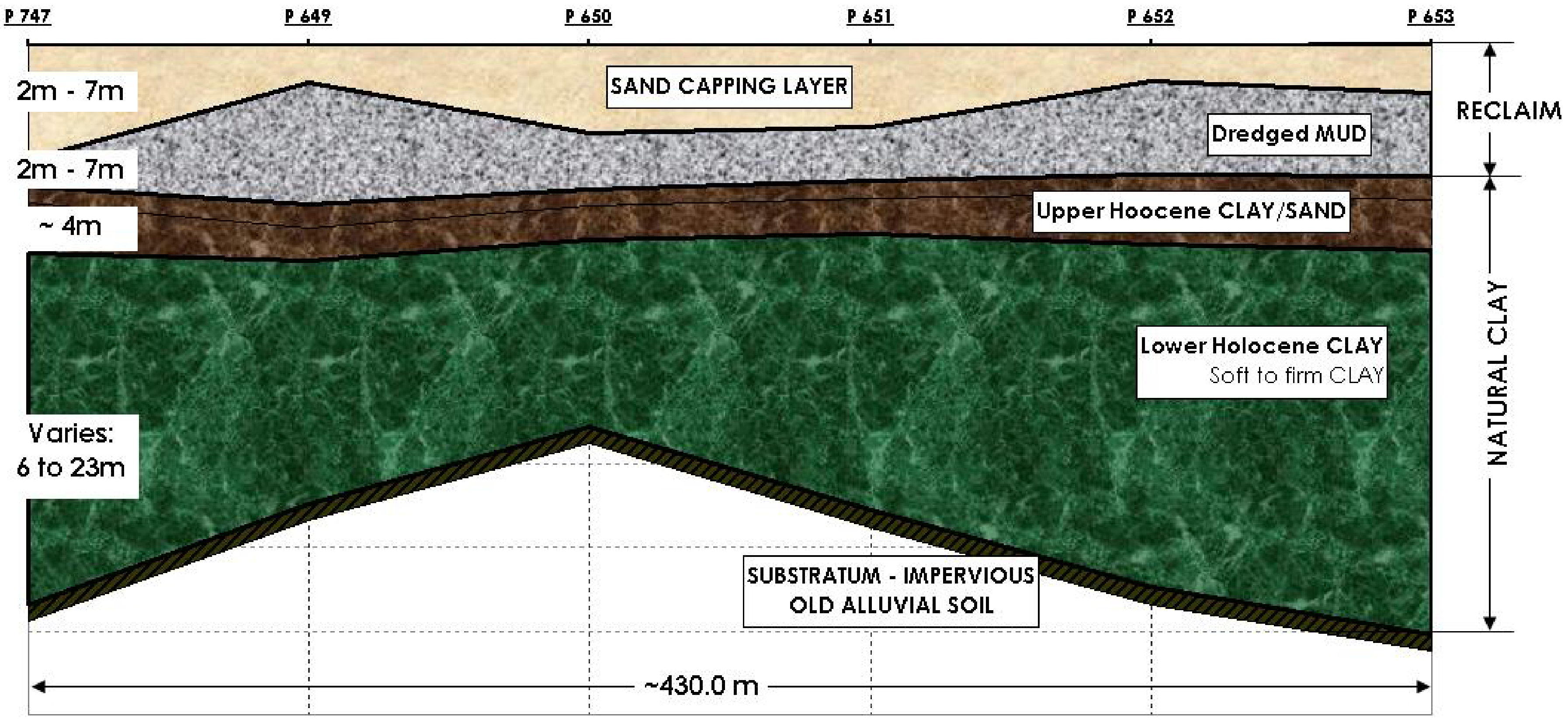
Case history – EADS Airbus Plant, Hamburg

PORT OF BRISBANE – PADDOCK S3B

PROJECT OVERVIEW

- **Located at the mouth of the Brisbane river;**
- **New reclamation area:** 234 ha **enclosed in the Port Expansion Seawall;**
- **Part of the new reclaimed area to be ready in 5years;**
- **Seawall construction completed in 2005;**

- ## GEOTECHNICAL LONG SECTION



GEOTECHNICAL LONG SECTION
SCALE N.T.S

GEOLOGICAL PARAMETERS

Parameter	Unit	Dredged Material	Upper Holocene Sand	Upper Holocene Clay	Lower Holocene Clay
$C_c/(1+e_0)$	[-]	0,235	0,01	0,18	0,235
$C_\alpha/(1+e_0)$	[-]	0,0059	0,001	0,008	0,0076
γ	[kN/m^3]	14	19	16	16
C_v	[m^2/y]	1	10	10	0.9
C_h	[m^2/y]	1	10	10	1.8
S_u	[kPa]	4	-	20	28
S_u / σ'_v	[-]	0,25	0,3	0,3	0,2

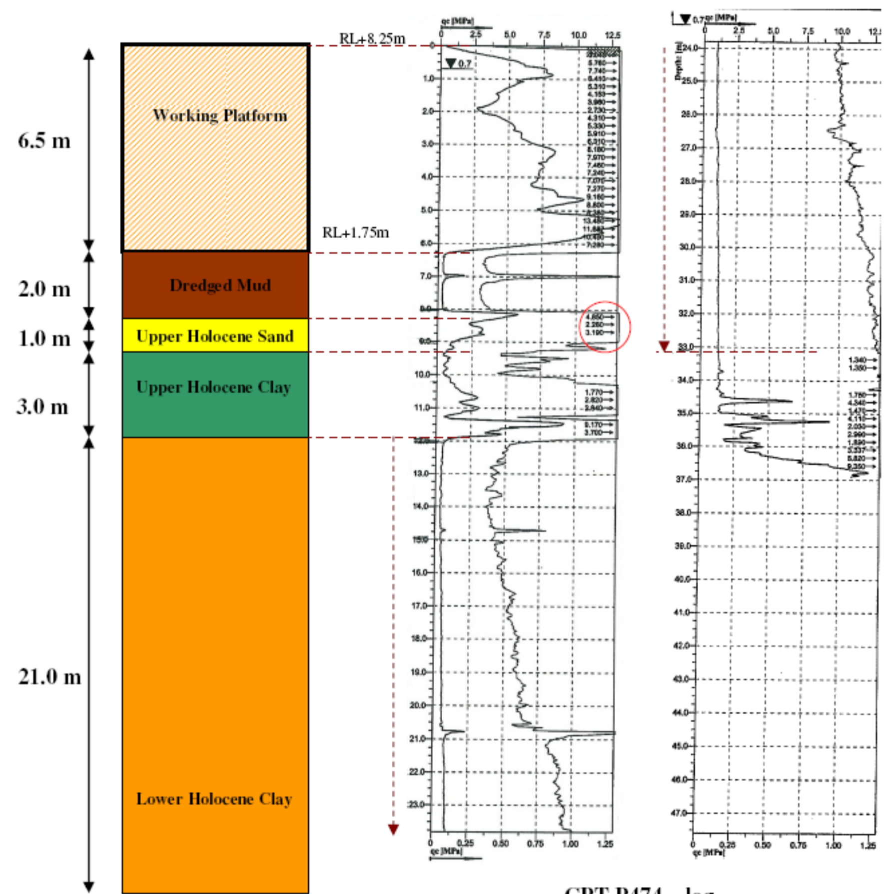
GEOLOGICAL SOIL PROFILE

AREA 2a

P474 location

- Water level during construction: RL+7.1m and RL+8.3m at vacuum start

- Working platform at RL+8.6m (thickness=6.8m) as of 22/12/08



Simplified Soil Profile for Consolidation Calculation

CPT P474 – log
As of 06/09/06

PORT OF BRISBANE – PADDOCK S3B

DESIGN CRITERIA & ASSUMPTIONS

Service Load:

Zone 1:	36kPa
Zone 2:	25kPa
Zone 3:	15kPa
Zone 4:	5kPa

Residual Settlement (20y):

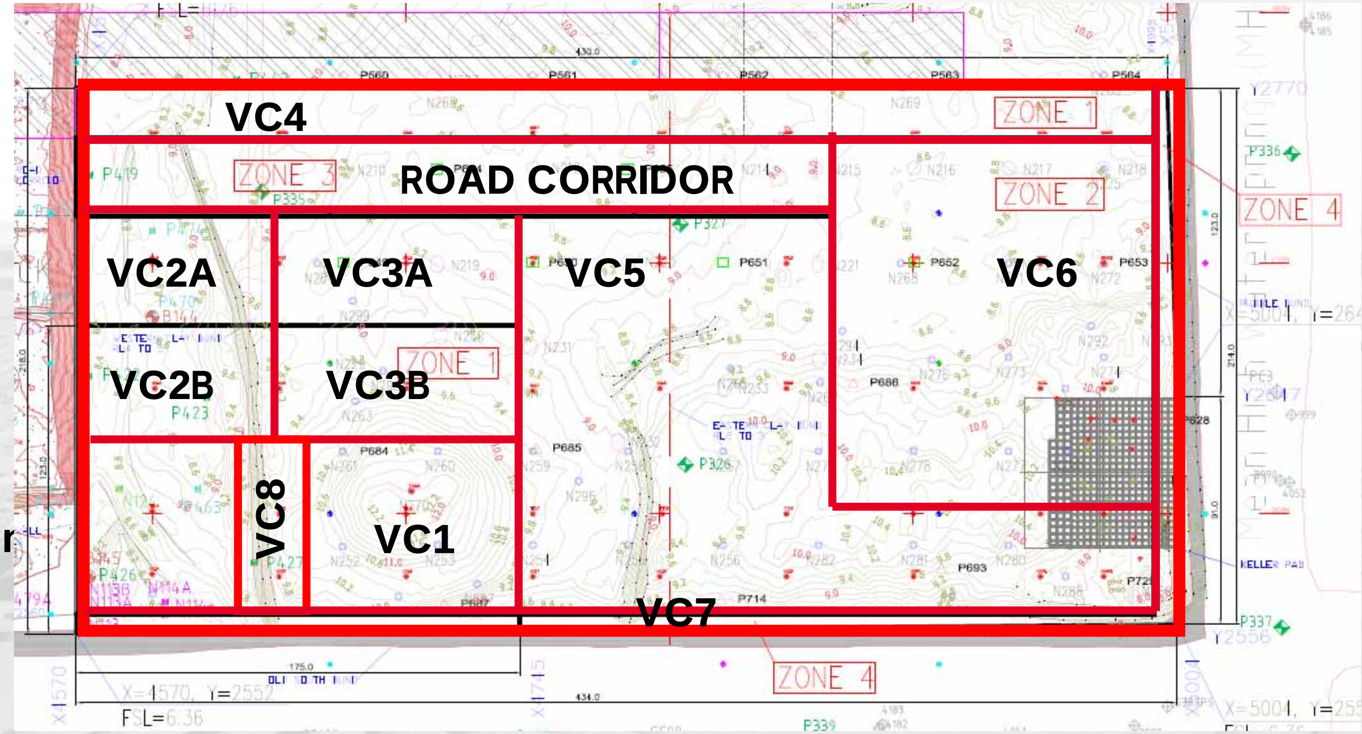
Zone 1 to 3:	150mm
Zone 4:	300mm

Vacuum pumping operation

18 months

Vacuum depressure:

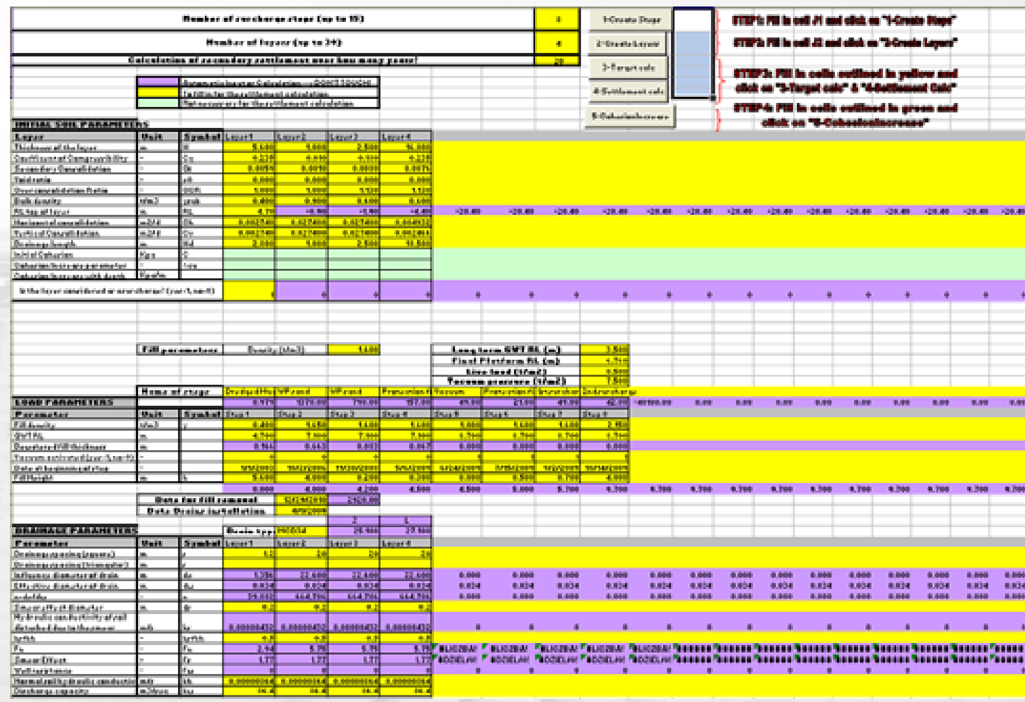
75.0 kPa

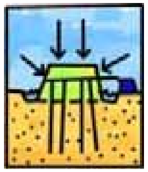


DESK STUDY – NUMERICAL ANALYSIS USING EXCEL SPREADSHEET *SETTLEMENT CALC.XLC*

Calculation of ***primary and secondary*** settlement;

- Secondary settlement to commence after primary settlement;
- Change in vertical stress is constant over the depth of the stratum;
- ***Buoyancy effect*** on the fill below the groundwater level due to settlement
- Fill to be removed instantaneous at the end of preloading period;
- Design load immediately applied at end of pre-loading period;



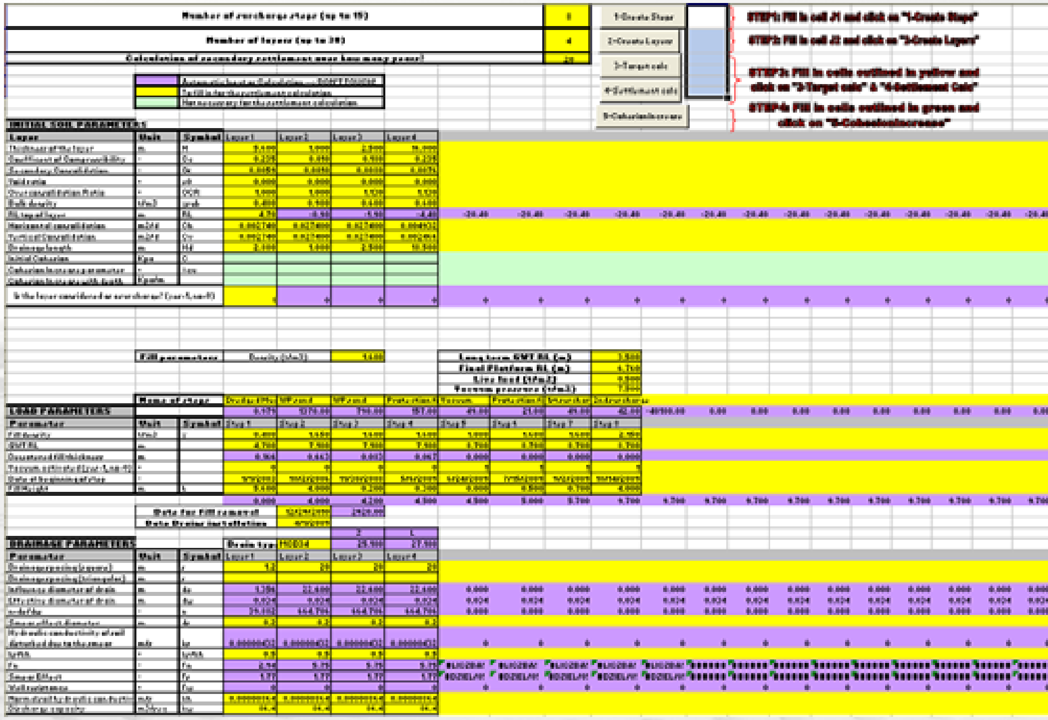


PORT OF BRISBANE – PADDOCK S3B

DESK STUDY…

- Up to 15 surcharge steps;
- Up to 30 soil layers;
- Calculation of *shear strength increase* during consolidation of cohesive soils;
- Different types of *drains* available:

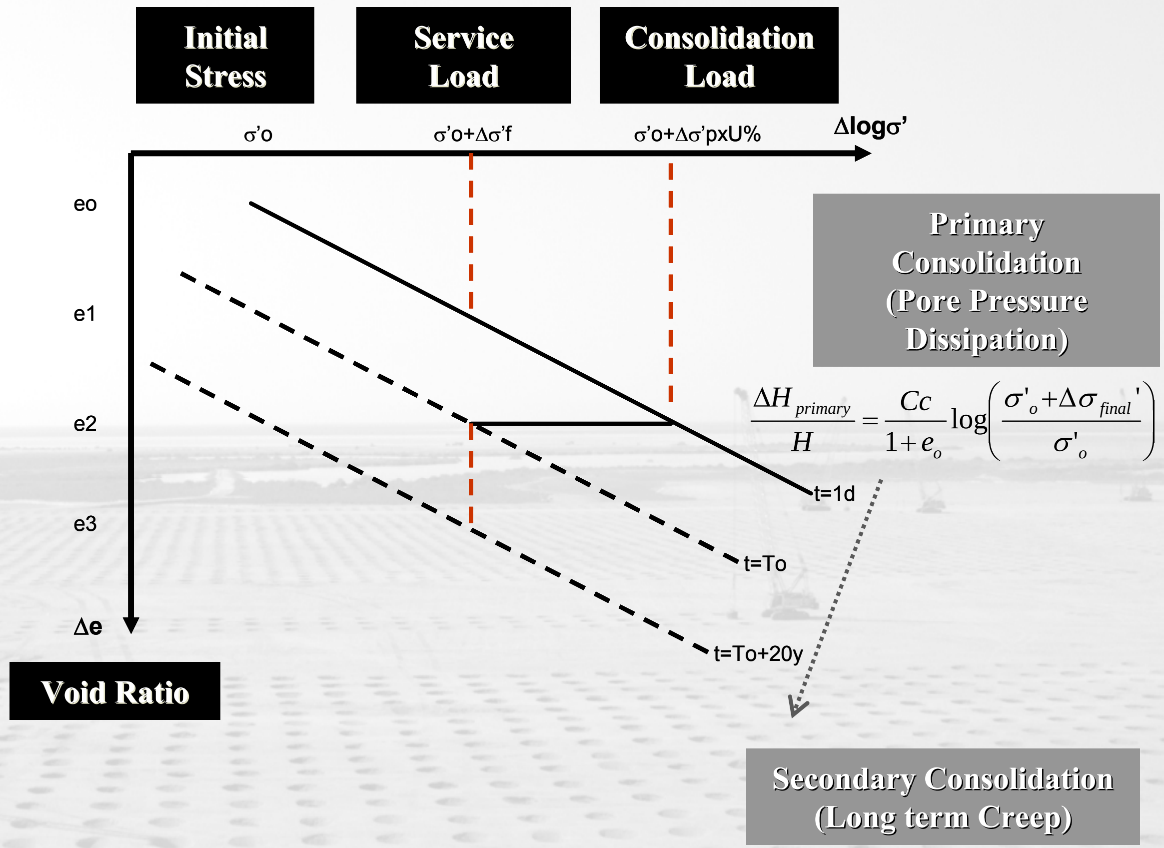
  MCD 34, MD88-3, FD767;
- *Effect of smear* due to mandrel insertion
- Graphical output Settlement / Fill thickness chart



ANALYSIS METHOD

- *Secondary Settlement*

Program uses a method based on Bjerrum's concept to calculate instantaneous and delayed consolidation (Bjerrum, 1967).

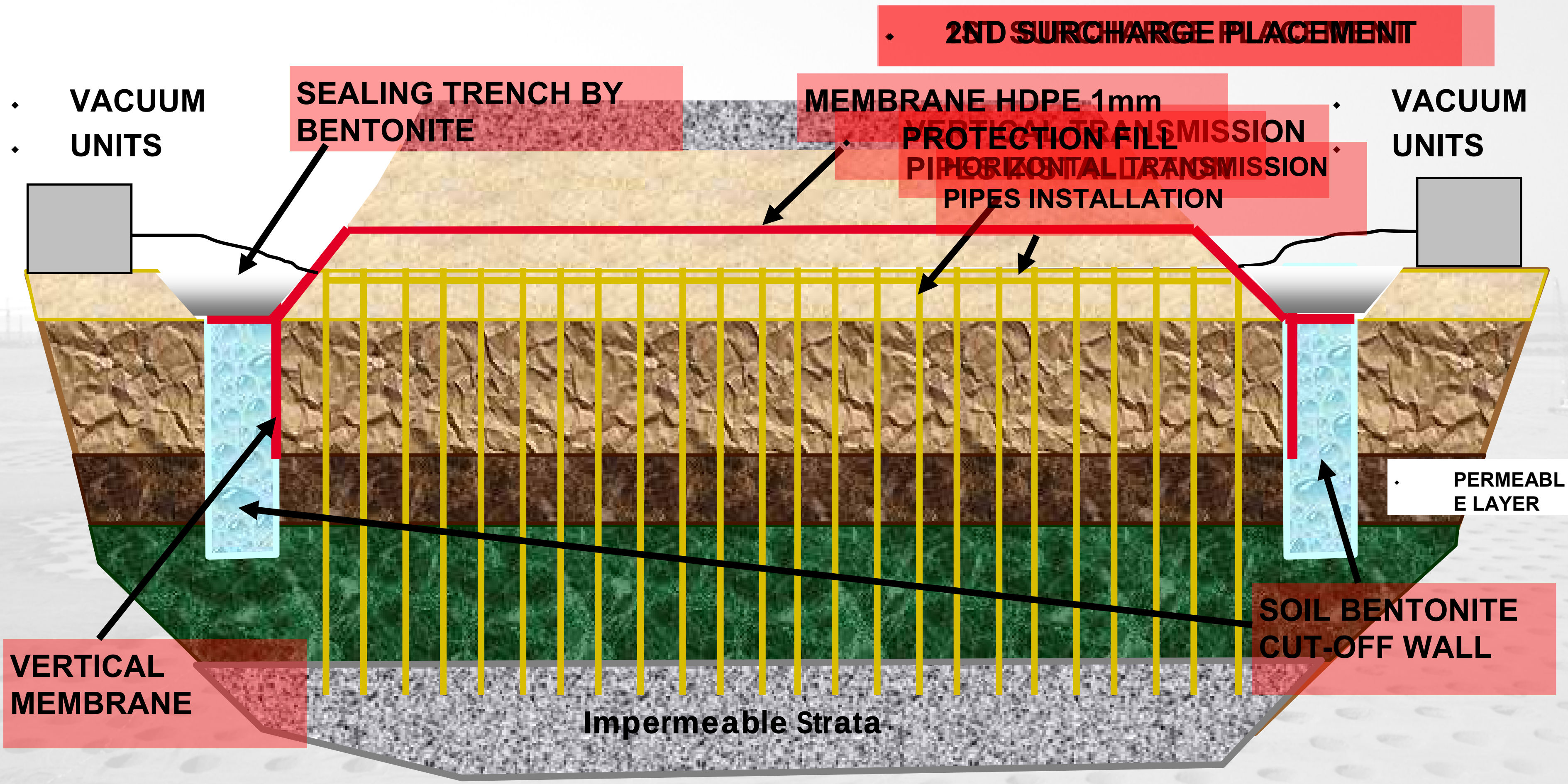


$$\frac{\Delta H_{primary}}{H} = \frac{Cc}{1+e_o}\log\left(\frac{\sigma'_o+\Delta\sigma_{final}'}{\sigma'_o}\right)$$

$$\Delta e = C_{\alpha e}\log\left(\frac{20\,years}{T_p}\right)$$

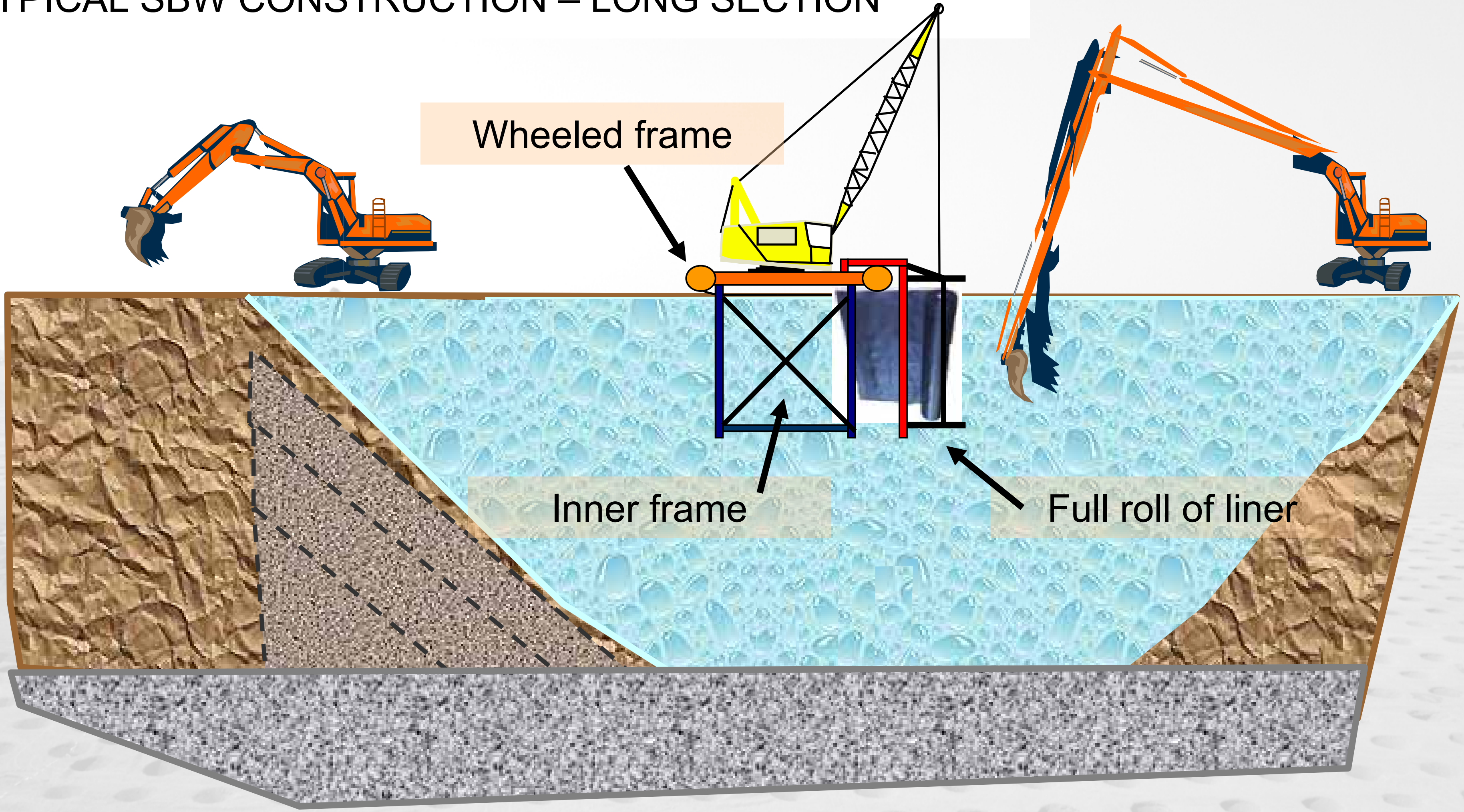
PORT OF BRISBANE – PADDOCK S3B

CONSTRUCTION SEQUENCE



PORT OF BRISBANE – PADDOCK S3B

TYPICAL SBW CONSTRUCTION – LONG SECTION



PORT OF BRISBANE – PADDOCK S3B

PORT OF BRISBANE – PADDOCK S3B

PORT OF BRISBANE – PADDOCK S3B

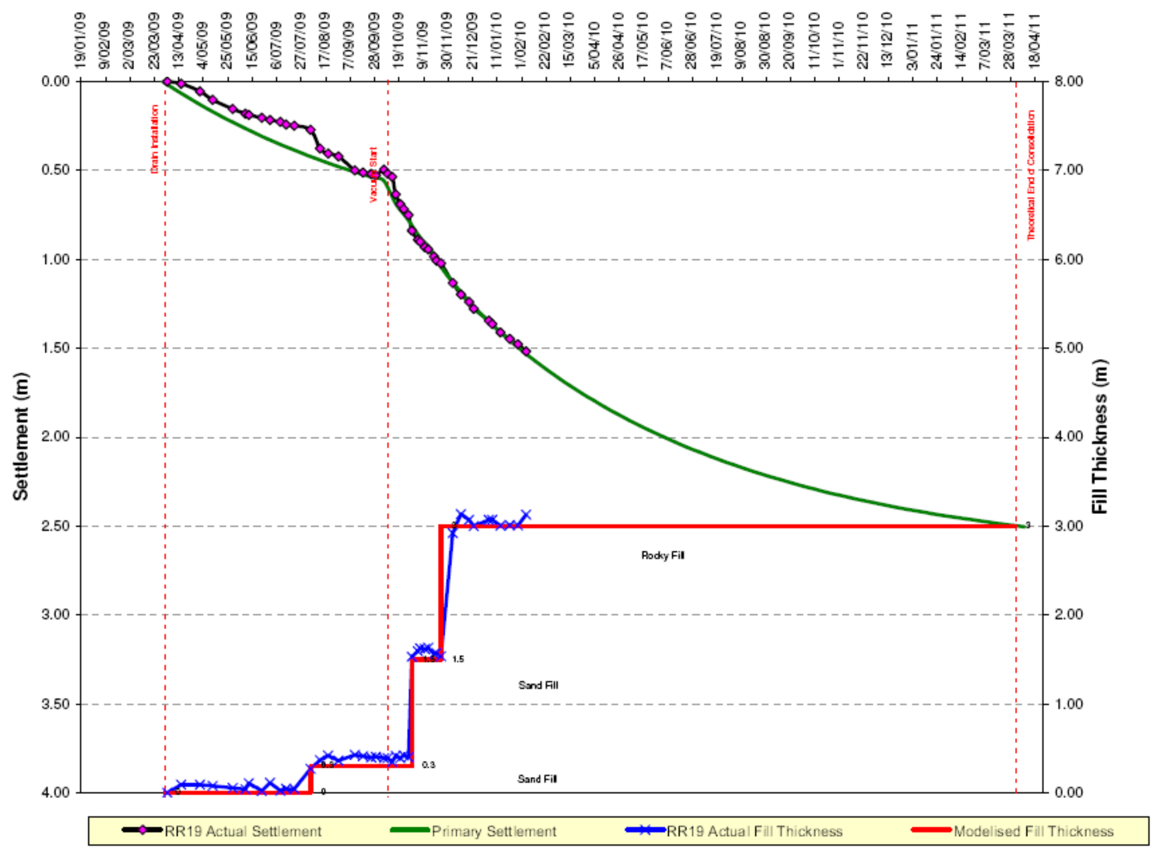
PORT OF BRISBANE – PADDOCK S3B

Project: **Vacuum Consolidation of Paddock S3B**
Proj. No.: **5040101**
Section: **RR19**

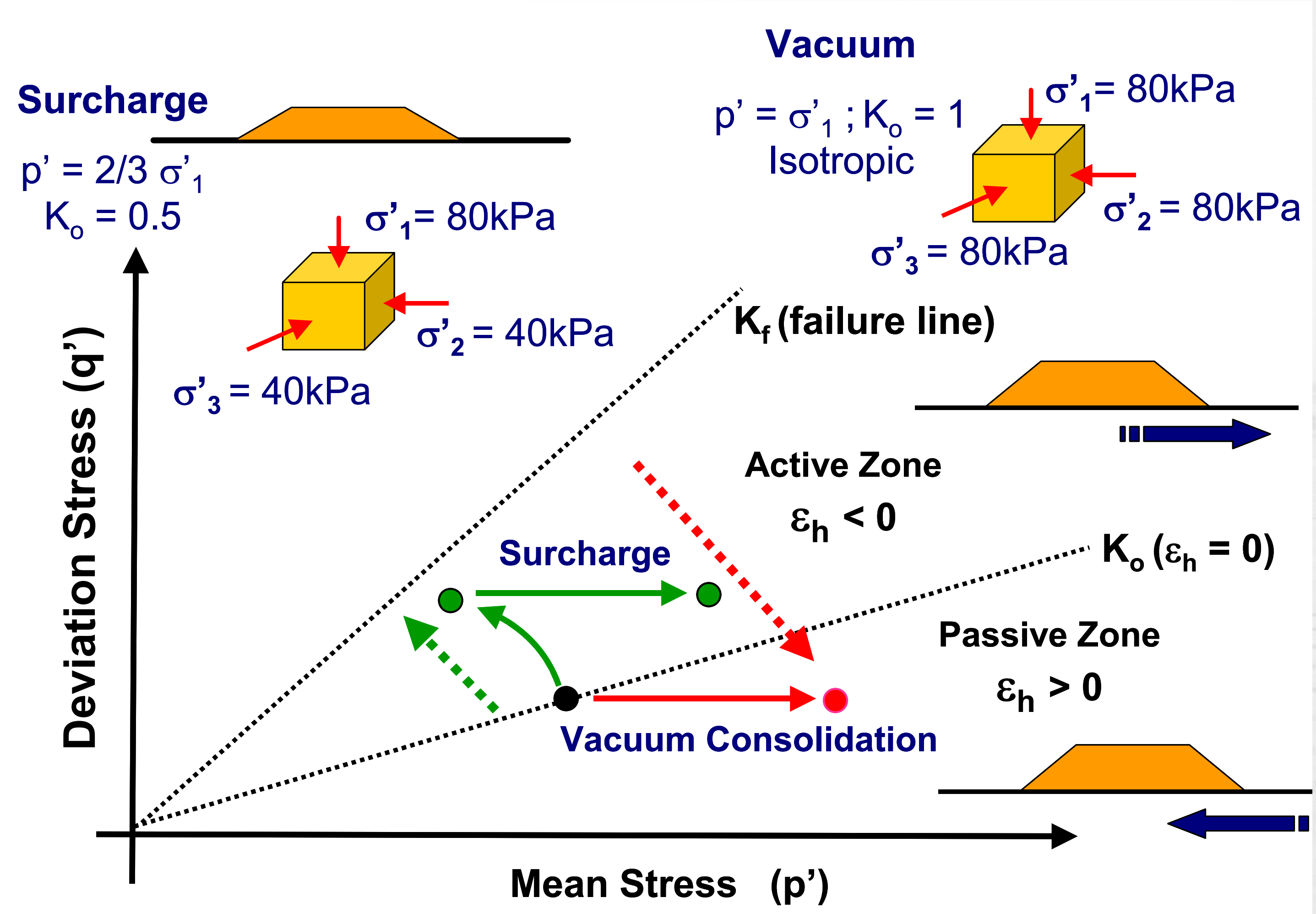
SIMPLIFIELD SOIL PROFILE

WATER LEVEL: **R.L. +7.10**
DURING PUMPING: **R.L. +8.80**
WORKING PLATFORM: **R.L. +9.00**
SOIL PROFILE: CPT: **P651**

SETTLEMENT / FILL THICKNESS

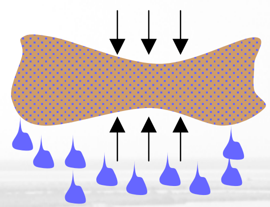
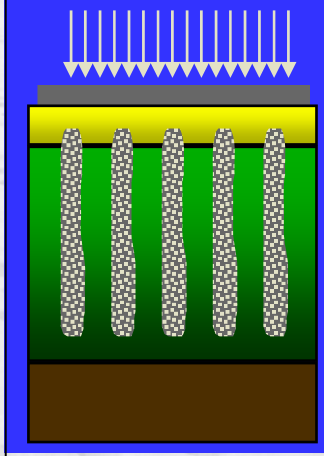
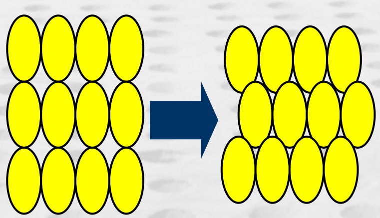


Stress path for Vacuum Process



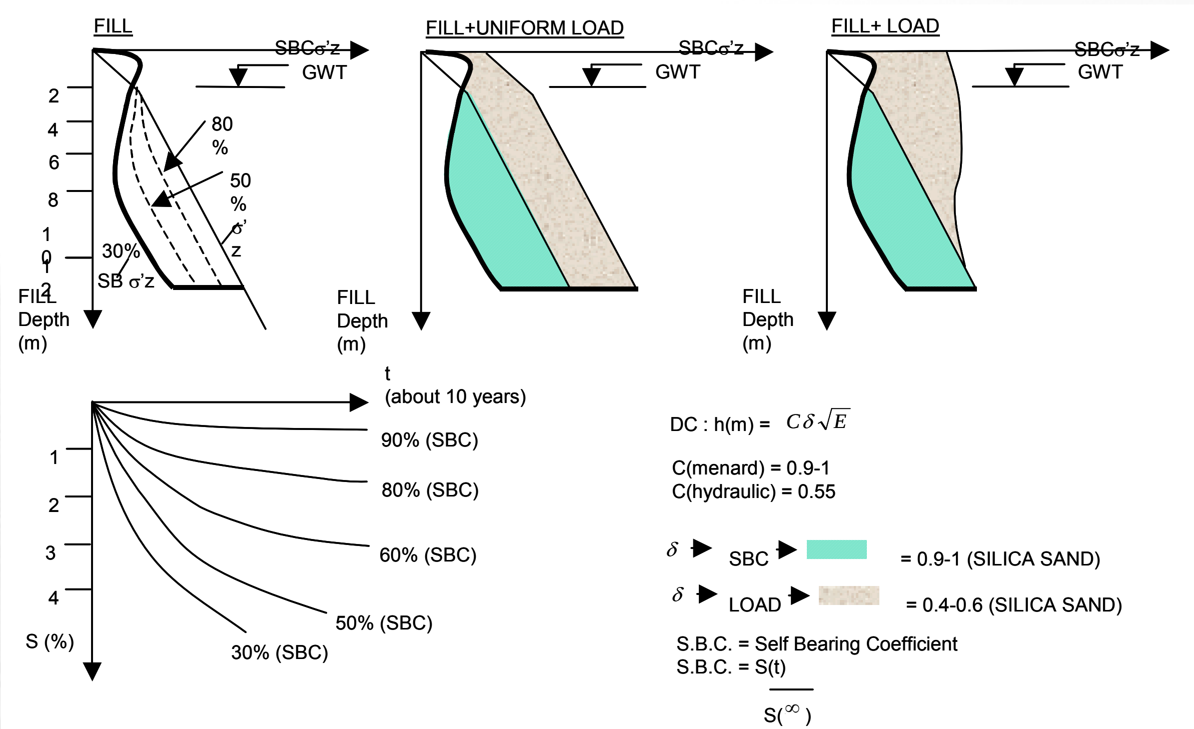
Case history : Kimhae (Korea) - 1998

Soil Improvement Techniques

	Without added materials	With added materials
Cohesive soil **Peat , clay …**	1 Drainage 2 VAcuum 	4 Dynamic replacement 5 Stone columns 6 CMC 7 Jet Grouting 8 Cement Mixing 
Soil with friction **Sand , fill**	3 Dynamic consolidation 4 Vibroflottation 	

Parameters for Concept

CONCEPT



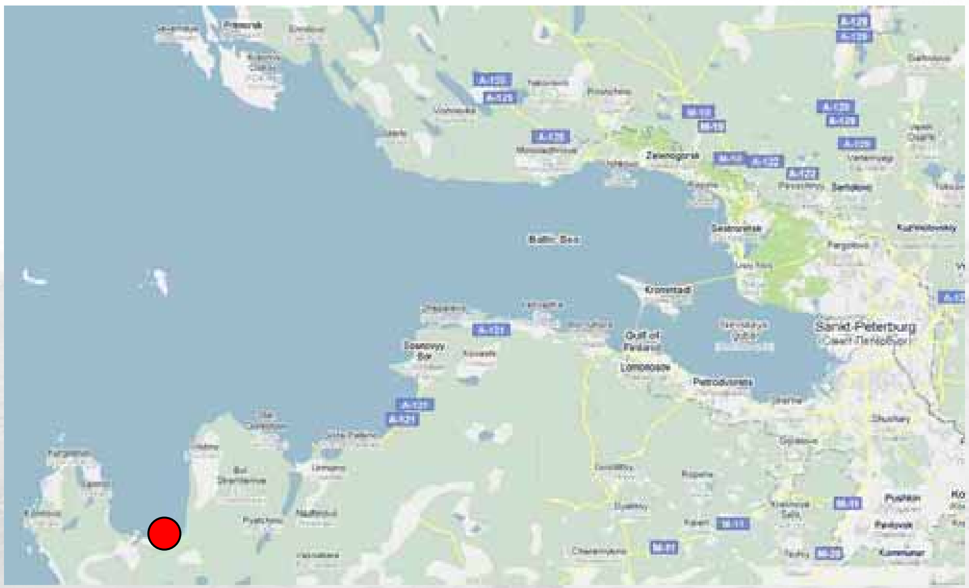
PARAMETERS

- Age if fill saturated or not
- P_L
- Selfbearing level
- ∅
- E_P or E_M
- Q_C, F_R,
- N
- R.D. (???)
- Shear wave velocity
- Seismic parameters
- Grain size

Nice airport runway consolidation
Granular soil

Very high energy (250 t , 40 m)

Site localisation - Место объекта



• 150 km from Saint-Petersburg

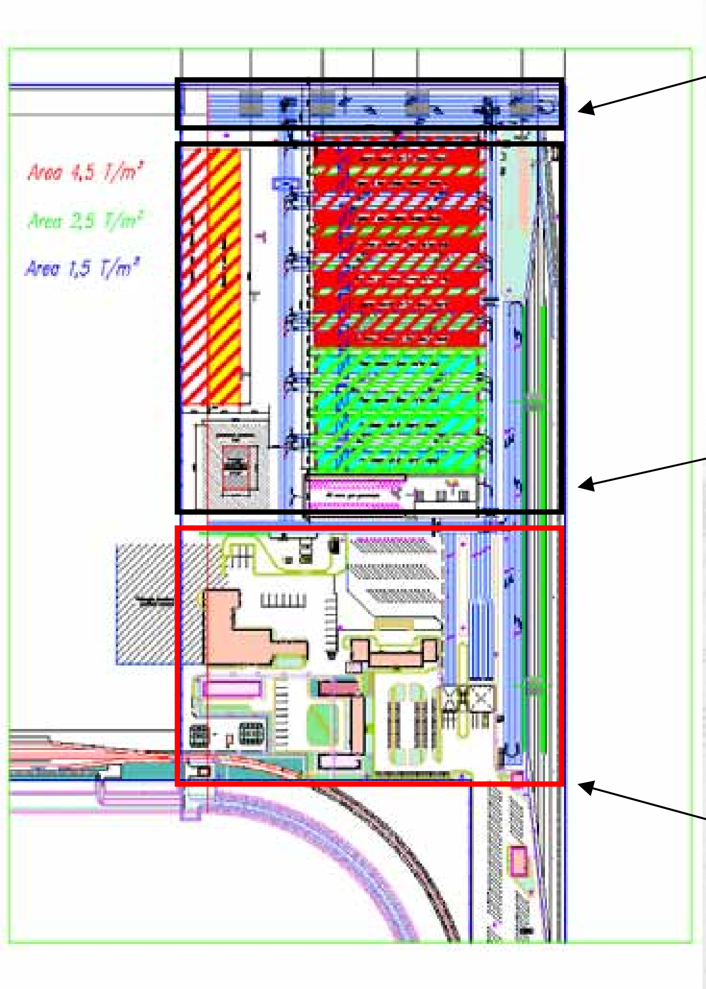
(150 км от Санкт-Петербурга)

• Main Port development program on the Baltic Sea

(Самый важний объект порта на Балтиском море)

Site overview - Вид объекта

Plan view - схема расположения



- heavy loads (тяжелая нагрузка)
- 6m of sand fill (6м песка)
- along the quay wall (вдоль причала)

=> Vibroflotation (виброфлотация)

- heavy loads (тяжелая нагрузка)
- 3 to 6m of sand fill (от 3 до 6м песка)

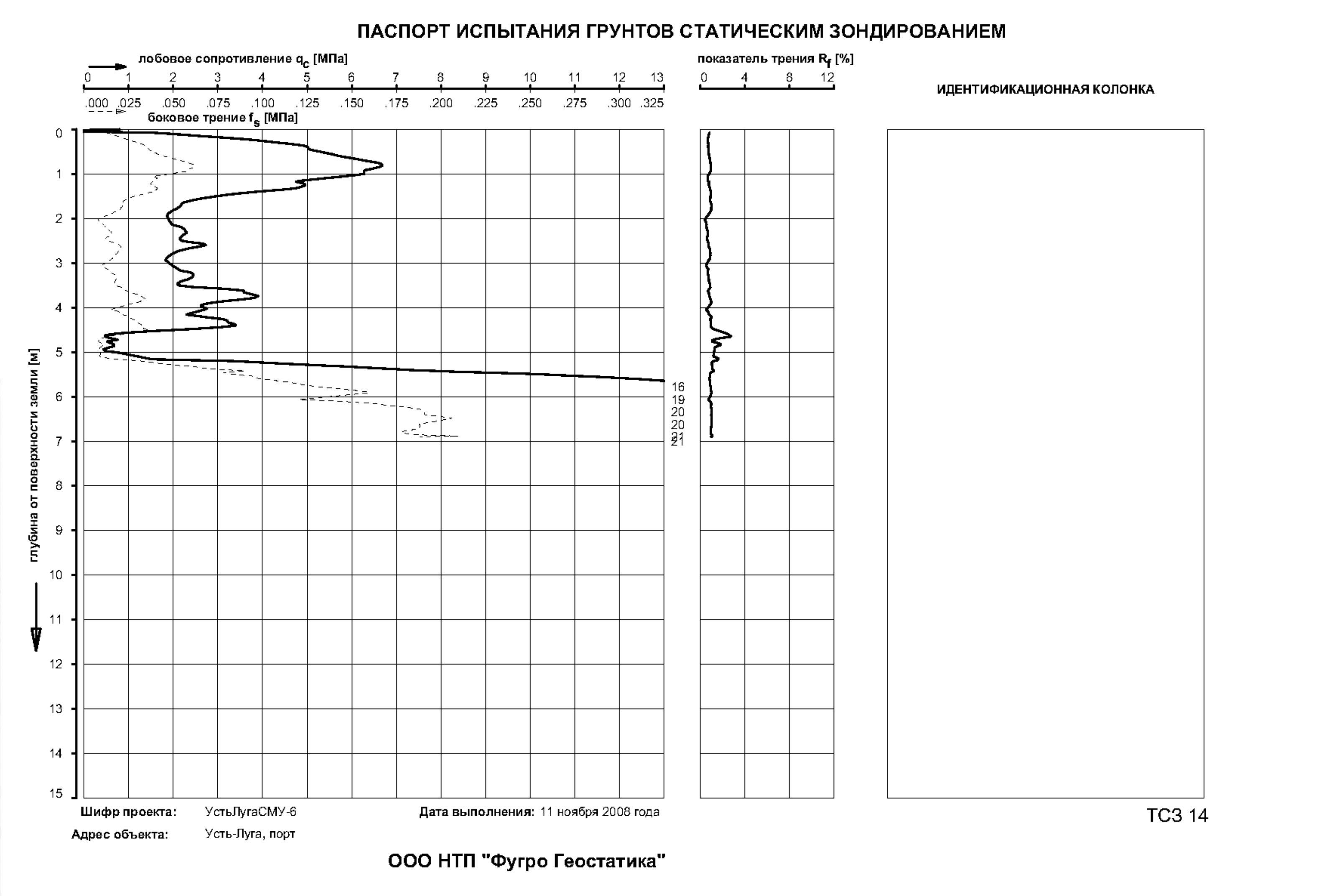
=> Dynamic Compaction (Динамическое Уплотнение)

- light loads (малинкая нагрузка)
- less than 3m of sand fill (меньшее 3м песка)

=> No compaction foreseen (ничего)

Few pictures before works
(несколько картини до работы)

Initial soil conditions : C.P.T.
(геологическая условия до работы)

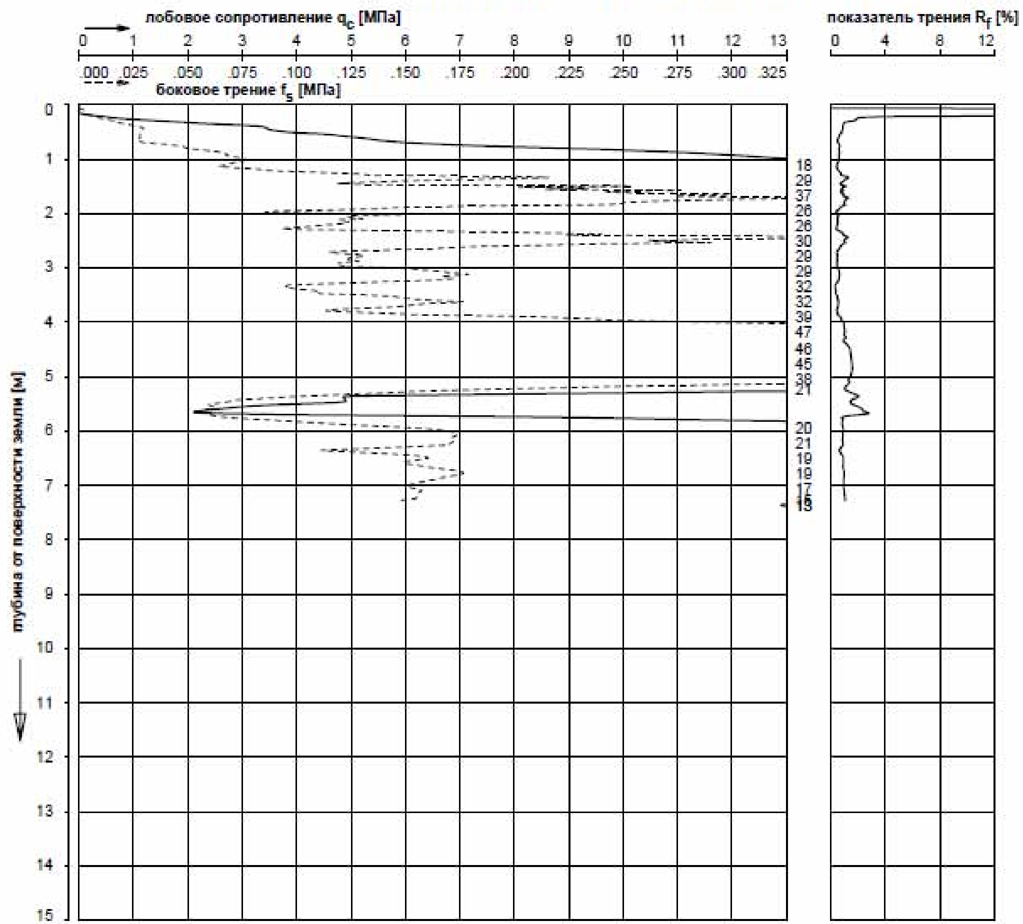


Few pictures of the ongoing works : Vibroflotation
(Картини от Виброфлотации)

Few pictures of the ongoing works : DC
(Картини от ДУ)

Results after compaction : Vibroflotation
(Результат после Виброфлотации)

ПАСПОРТ ИСПЫТАНИЯ ГРУНТОВ СТАТИЧЕСКИМ ЗОНДИРОВАНИЕМ



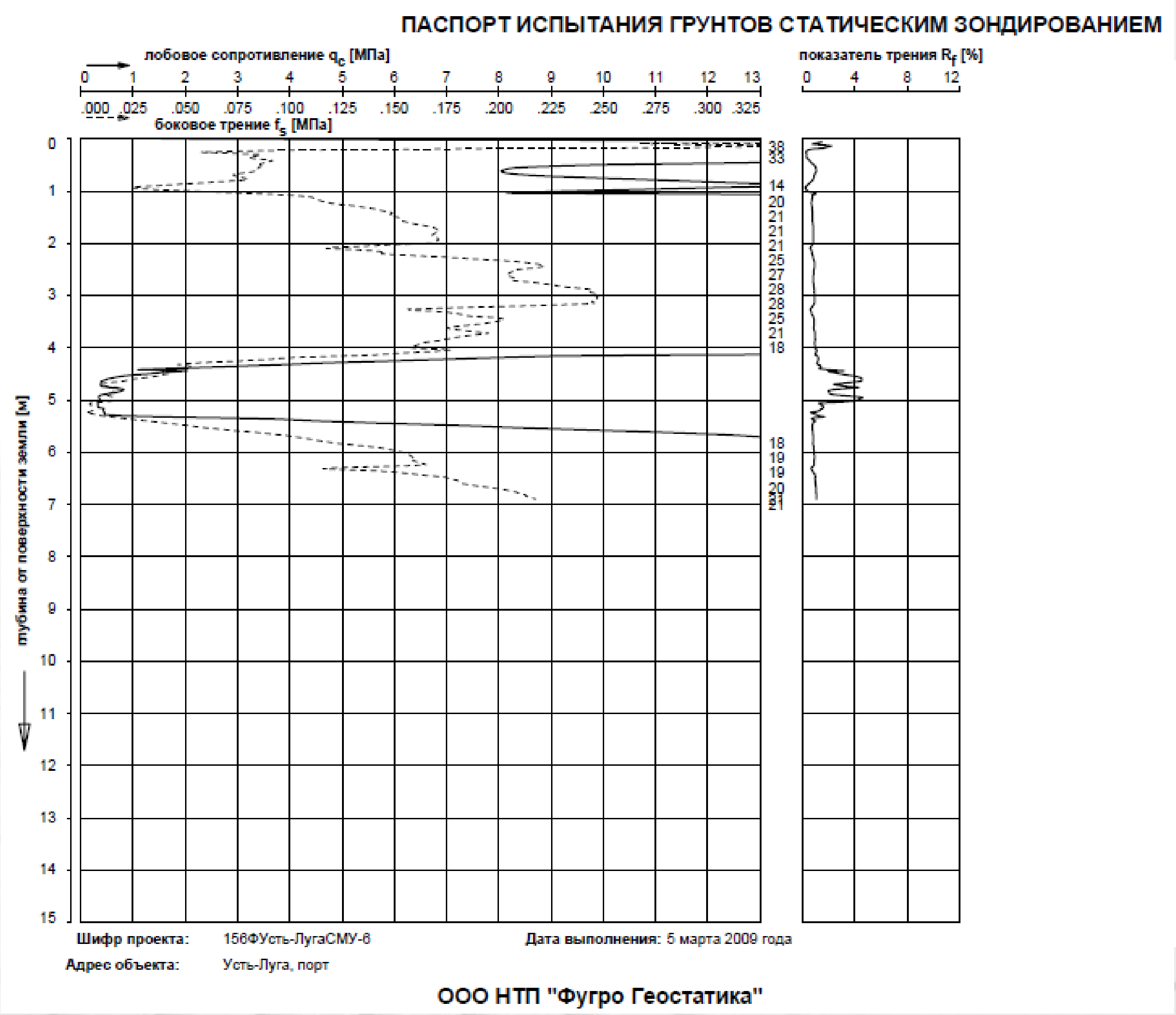
Шифр проекта: 156фУстьЛуга

Дата выполнения: 1 декабря 2008 года

Адрес объекта: Усть-Луга, порт

ООО НТП "Фугро Геостатика"

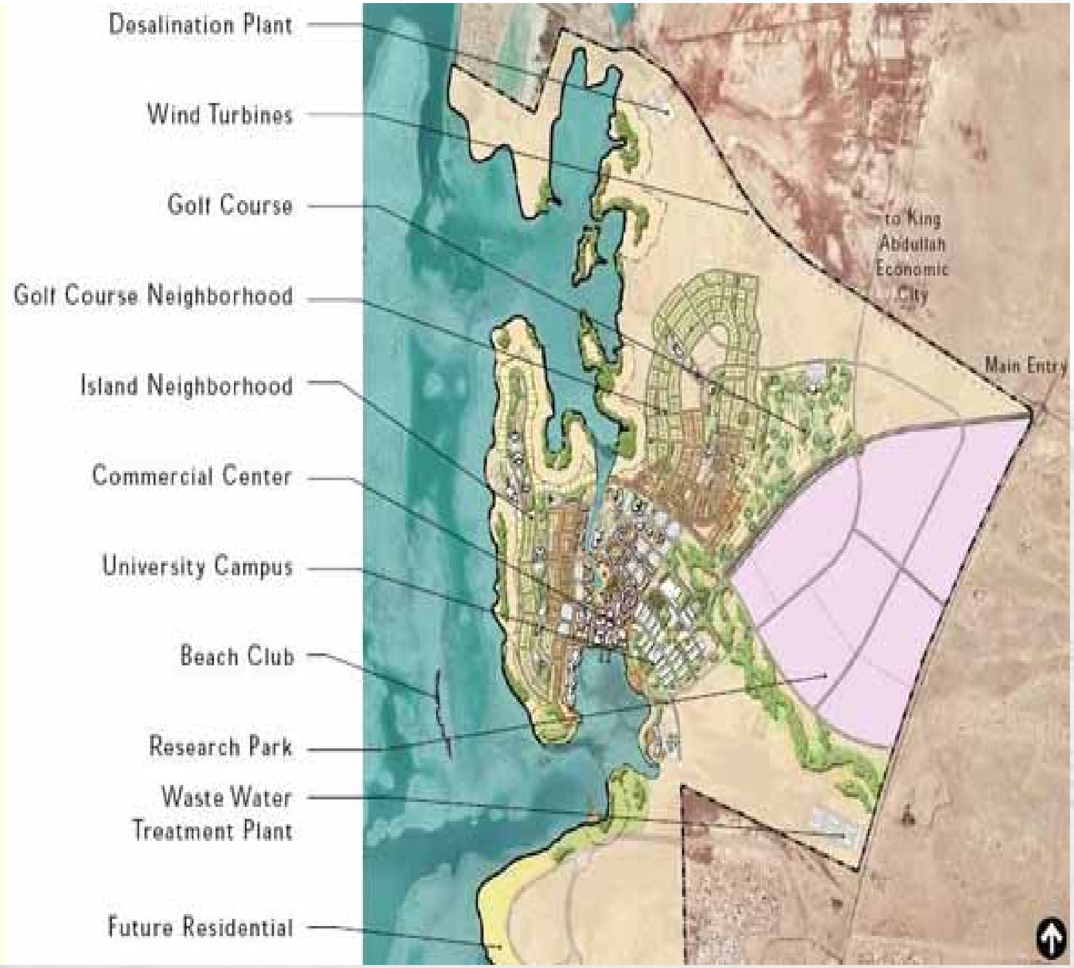
Results after compaction : Dynamic Compaction
(Результат после Динамического Уплотнения)



Concept and application of ground improvement for a 2,600,000 m²

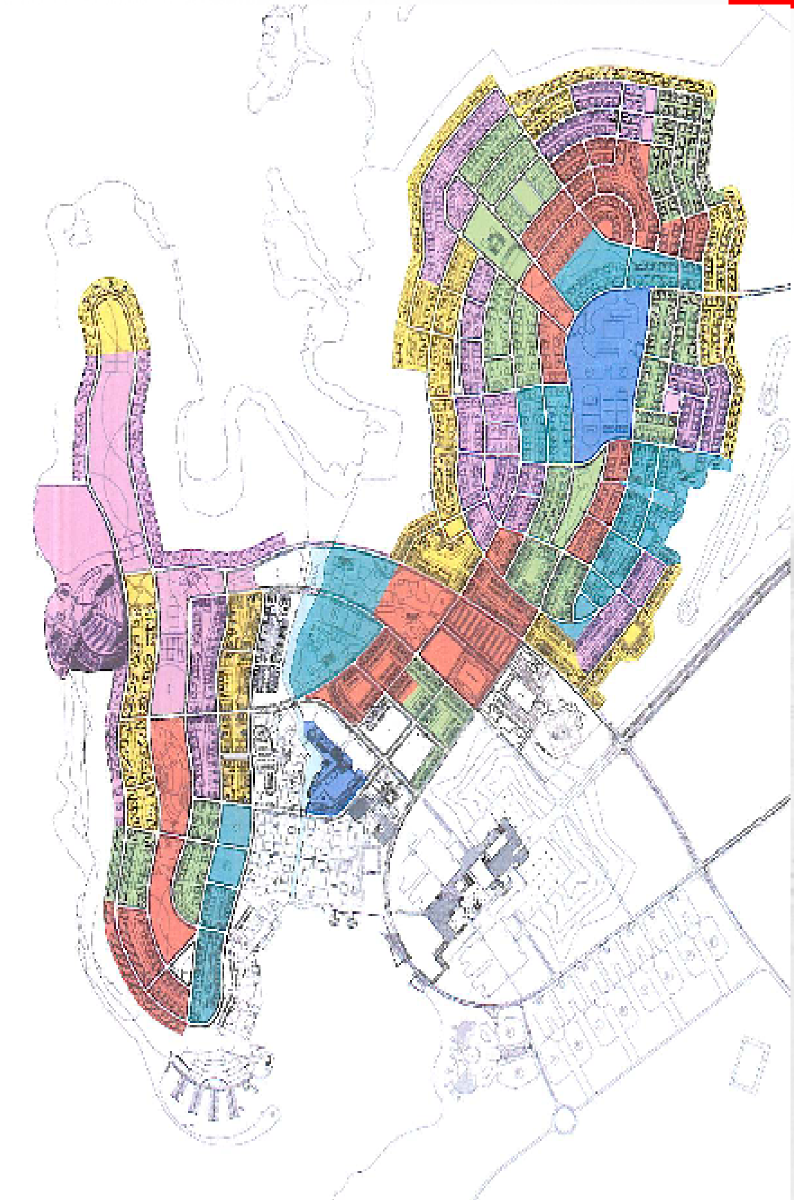
FUTURE UNIVERSITY CAMPUS

Typical Master Plan



Discovering the Habitants

Areas to be treated



AREAS TO BE TREATED

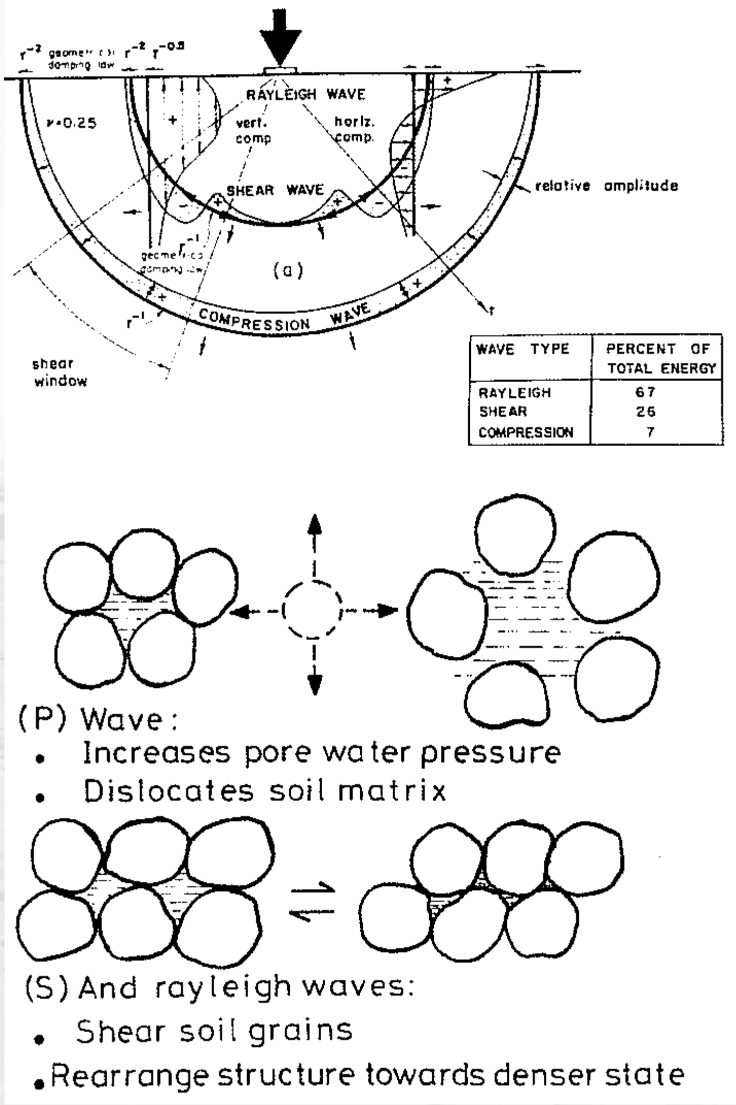
- AL KHODARI (1.800.000 m2)
- BIN LADIN (720.000 m2)

SCHEDULE

- 8 month

Dynamic Consolidation

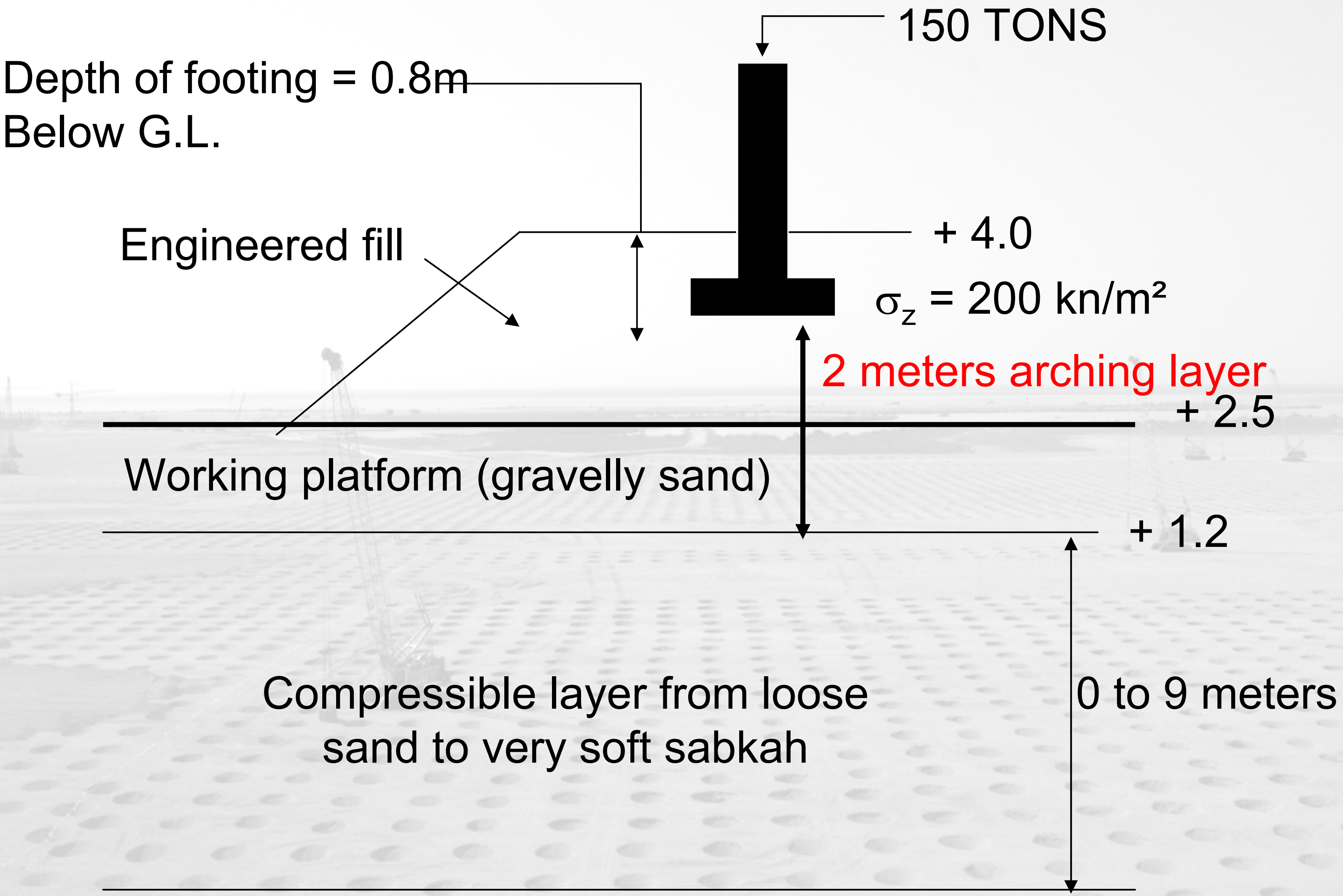
Shock waves during dynamic consolidation – upper part of figure after R.D. Woods (1968).



Specifications

- Isolated footings up to 150 tons
- Bearing capacity 200 kPa
- Maximum footing settlement 25 mm
- Maximum differential settlement 1/500
- Footing location unknown at works stage

Concept


150 TONS
Depth of footing = 0.8m
Below G.L.
Engineered fill
+ 4.0
σ_z = 200 kn/m²
2 meters arching layer
+ 2.5
Working platform (gravelly sand)
+ 1.2
Compressible layer from loose
sand to very soft sabkah
0 to 9 meters

$$(D) = C\,\delta\sqrt{WH}$$

where: C is the type of drop. Its value is given in Table.
δ is a correction factor. δ = 0.9 for metastable soils, young fills, or very recent hydraulic fills and δ = 0.4 – 0.6 for sands.

Table Values of coefficient C in the equation

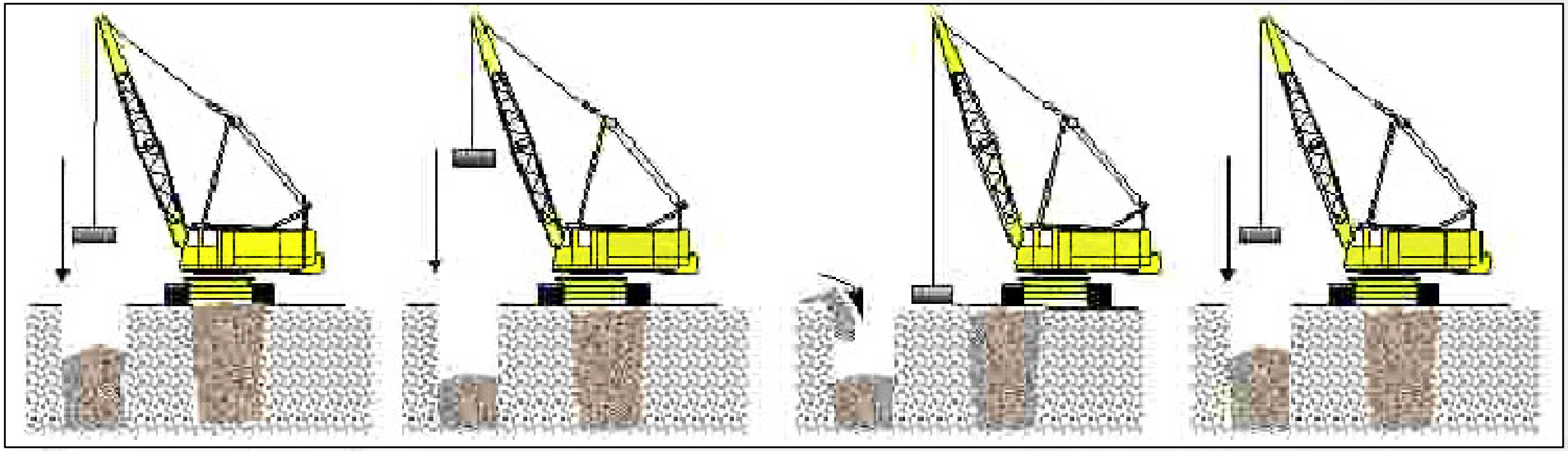
Drop method	Free drop	Rig drop	Mechanical winch	Hydraulic winch	Double hydraulic winch
C	1.0	0.89	0.75	0.64	0.5

The equation has been revised recently by Varaksin and Racinais (2009) as:

$$f(z) = \frac{f_2 - f_1}{D^2}(z - NGL)^2 + f_1$$

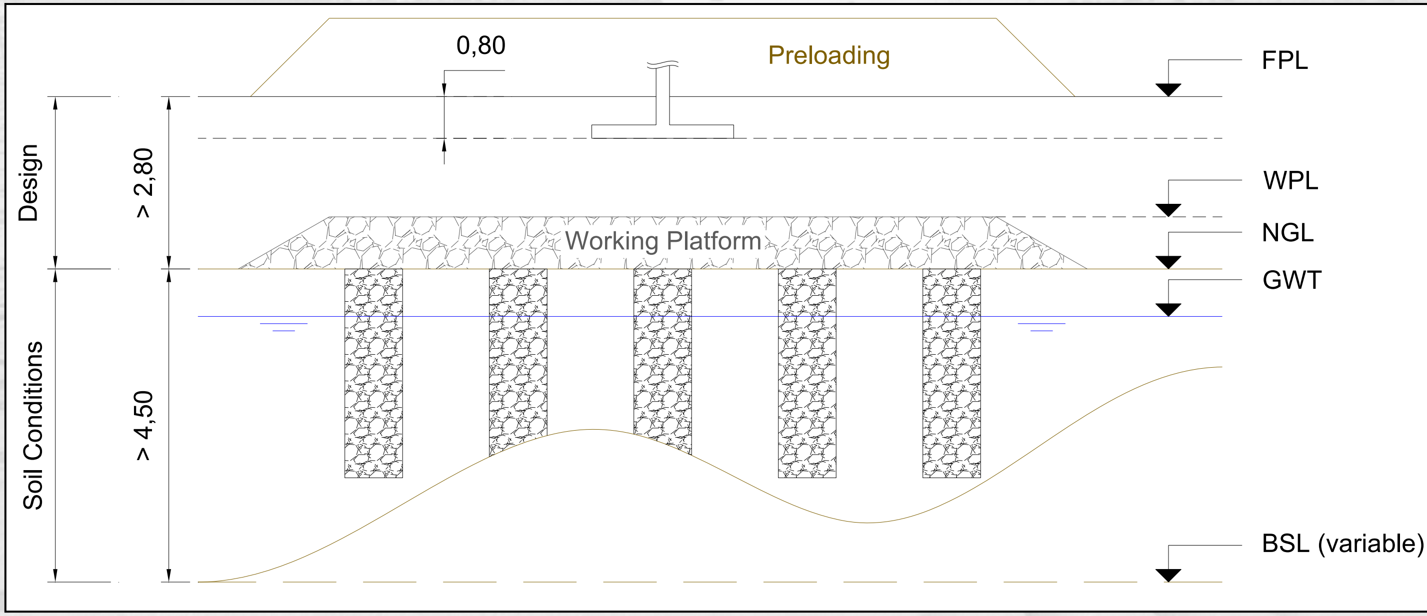
Where: f(z) is the improvement ratio at elevation (z); z is the depth in meters; NGL is the natural ground level; D is the depth of influence of dynamic consolidation; f_1 is the maximum improvement ratio observed at ground surface and it is dimensionless. The value may be taken as f_1 = 0.008E and E is the energy in tons-meter/m^2; and f_2 is the improvement ratio at the maximum depth of influence that can be achieved.

Selection of technique

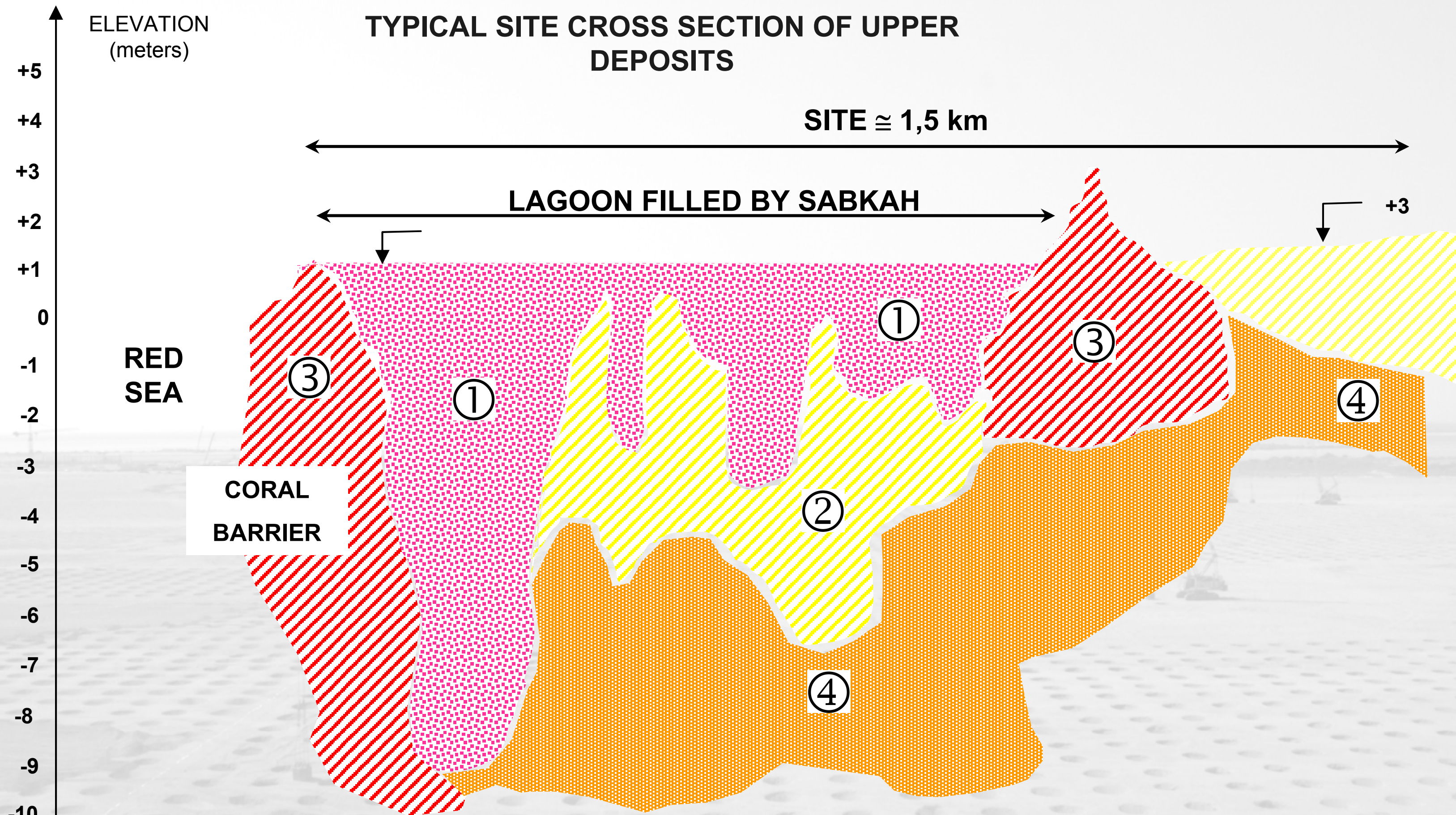


DR (Dynamic Replacement)

HDR (High Energy Dynamic Replacement) + surcharge

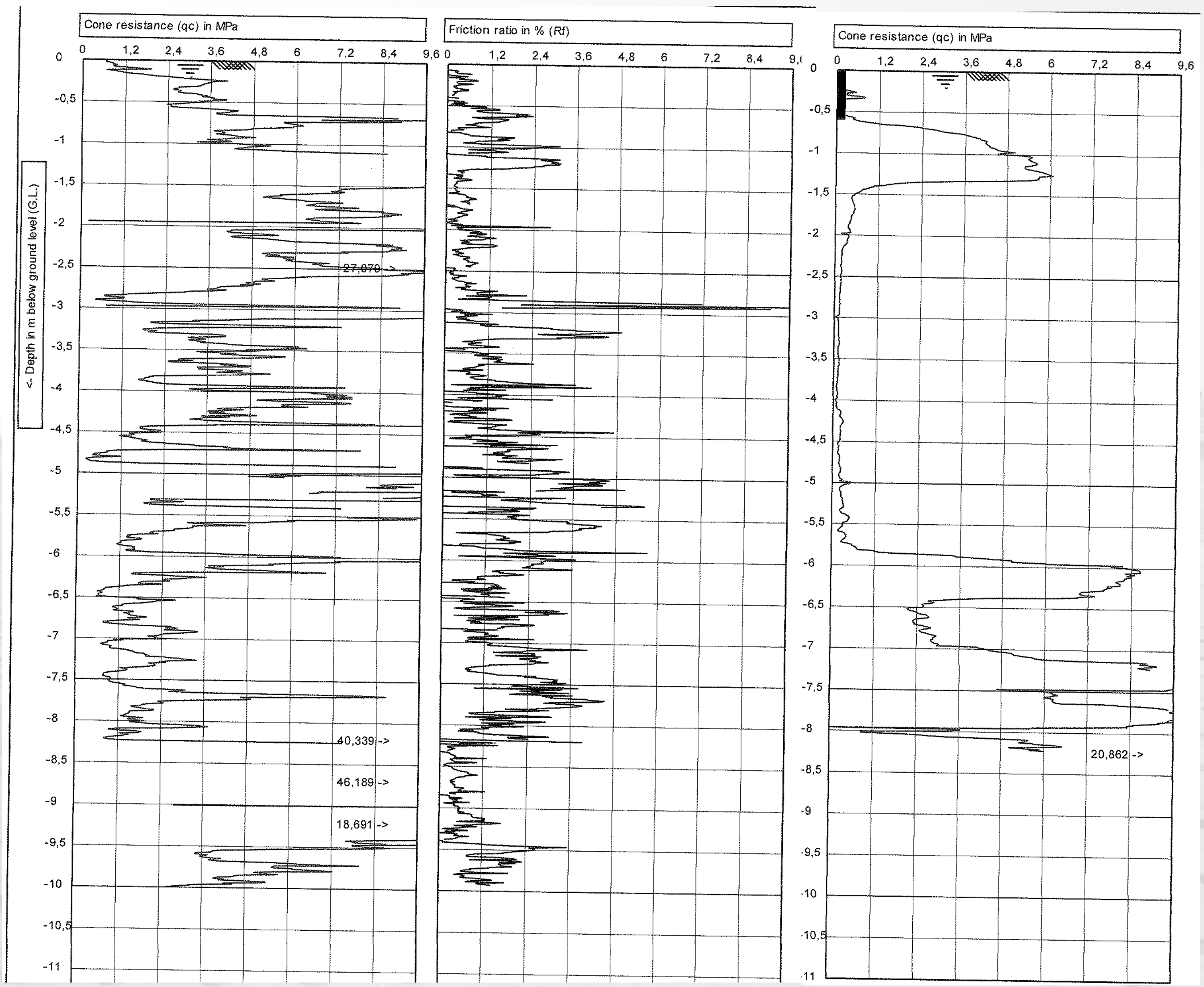


Specifications



LAYER	USC	w %	% fines	N	Qc BARS	F_R %	P_L BARS	E_P BARS
1 - SABKAH	SM + ML	35-48	28-56	0-2	0-2	1,2-4	0,4-1,9	avr-17
2 - LOOSE SILTY SAND	SM	-	15-28	3-9	12-45	0,5-1,2	2,1-4	18-35
3 - CORAL	-	26-35	-	6-12	-	-	5,1-7,2	35-60
4 - LOOSE TO MED DENSE SAND	SM	-	12-37	3-18	15-80	0,5-1,8	4-12	28-85

Variation in soil profile over 30 meters

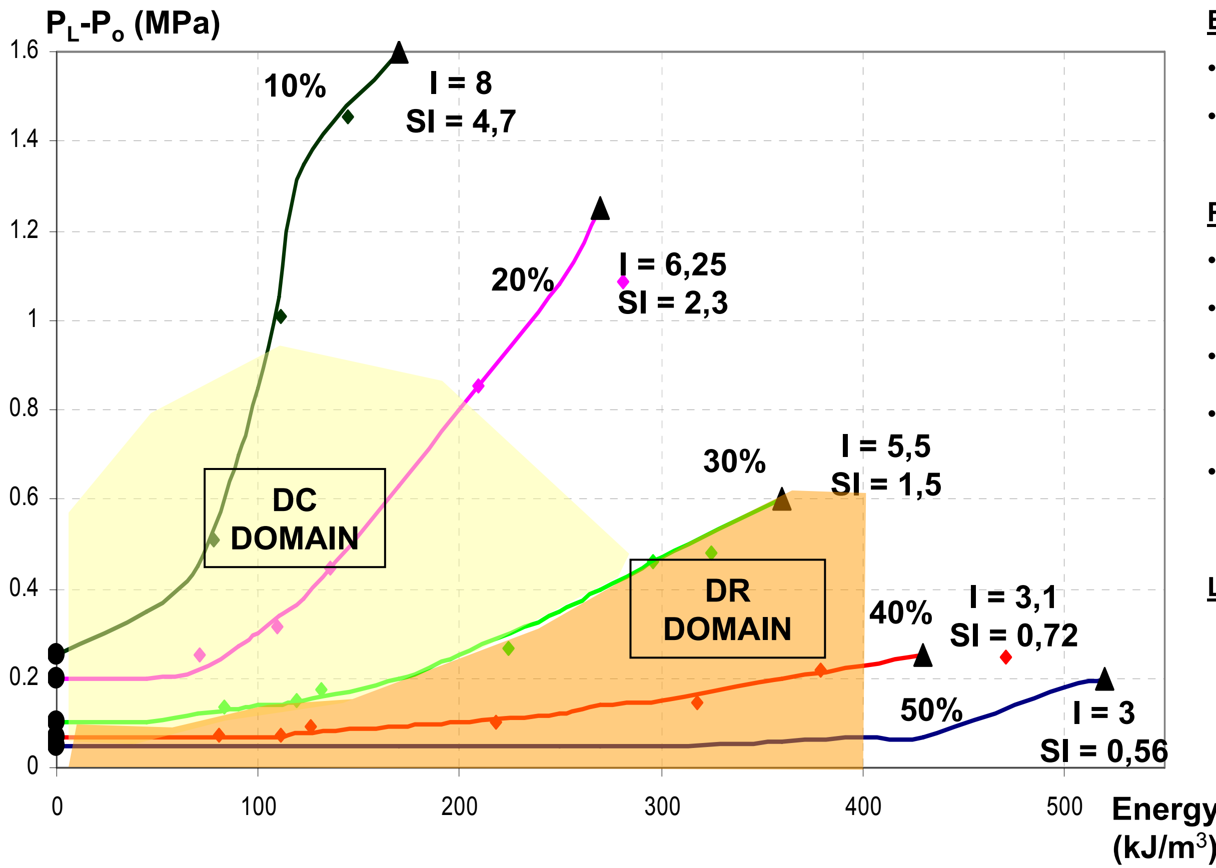


Typical surface conditions

Analysis of improvement

ANALYSIS OF (P_L-P_o) IMPROVEMENT AS FUNCTION OF ENERGY AND FINES

K.A.U.S.T. – Saudi Arabia



BASIS

- 60 grainsize tests
- 180 PMT tests

PARAMETERS

- $P_L - P_o$ = pressuremeter limit pressure
- kJ/m^3 = Energy per m^3 (E)
- % = % passing n°200 sieve
- I = improvement factor $\frac{P_{LF}}{P_{Li}}$
- S.I : energy specific improvement factor $\frac{I \times 100}{E}$

LEGEND

- ● Average pre-treatment values
- ◆ Average values between phases
- ▲ Average post-treatment values

It can be assumed that those impacts du generate a pore pressure at least equal to the pore pressure generated by the embankement load.

This new consolidation process with the final at a time t'_f, where

$$T_V = 0{,}848 = \frac{C'_v\,(t'_1 - t_1)}{H^2} + \frac{C_v T_1}{H^2}$$

With

$$C'_V = C_V\left[1 + \frac{du}{\Delta\sigma(1 - U_1)}\right]$$

The following equation allows to compare the respective times of consolidation being :

t'_f with impact

t_f without impact

$$t'f = \frac{du}{du + \Delta\sigma(1 - U_1)}\,t_1 + \frac{\Delta\sigma(1 - U_1)}{du + \Delta\sigma(1 - U_1)}\,t_f$$

For the considered case,

$$du = U\Delta\sigma$$

and thus $t'_f = U_1 t_1 + (1-U_1)t_f$

The Table allows to compare the gain in consolidation time, at different degrees of consolidation.

U_1	10%	20%	30%	40%	50%	60%	70%	80%	90%
t_1/t_f	0.009	0.037	0.083	0.148	0.231	0.337	0.474	0.669	1.00
t'_1/t_f	0.901	0.807	0.725	0.659	0.615	0.602	0.632	0.735	1.00

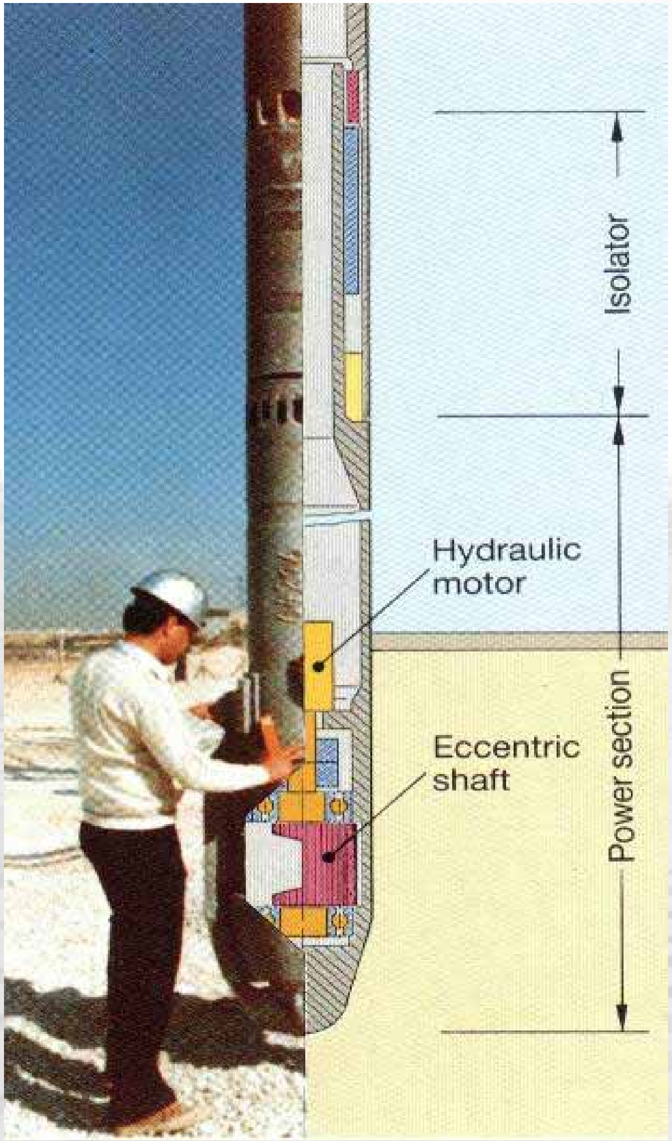
Supposing primary consolidation completed

$U = 0.9$ or $T = 0.848$ if $du = U_1\Delta\sigma$,
then $t'_f = U_1 t_1 + (1-U_1)t_f$

The optimal effectiveness occurs around $U_1 = 60\%$.
One can thus conclude that, theoretically the consolidation time is reduced by 20% to 50%, what is for practical purpose insufficient.

Dynamic surcharge

VIBROFLOTS

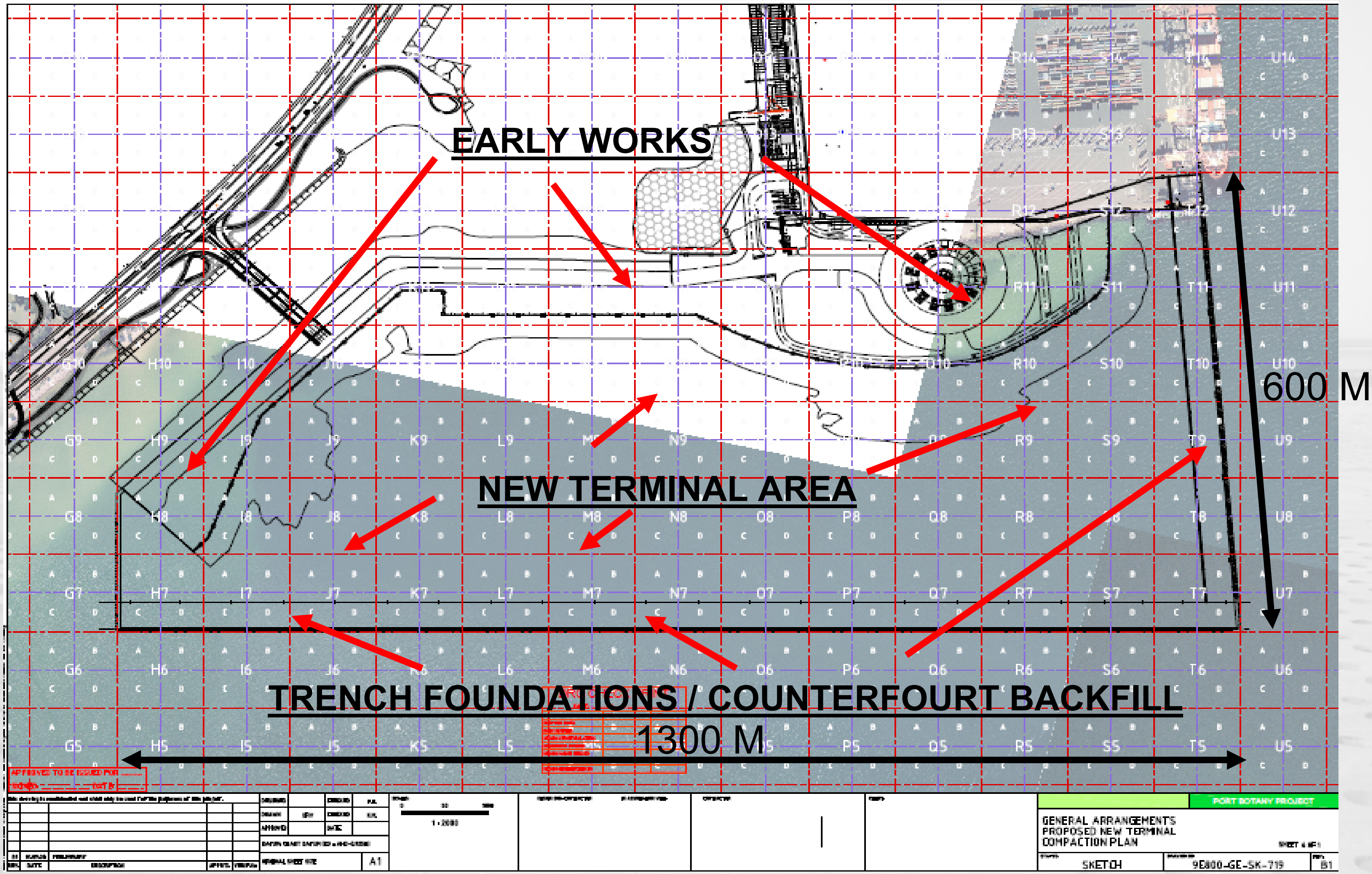

Isolator
Hydraulic motor
Eccentric shaft
Power section


Amplitude
28 – 48mm

GROUND COMPACTION WORKS

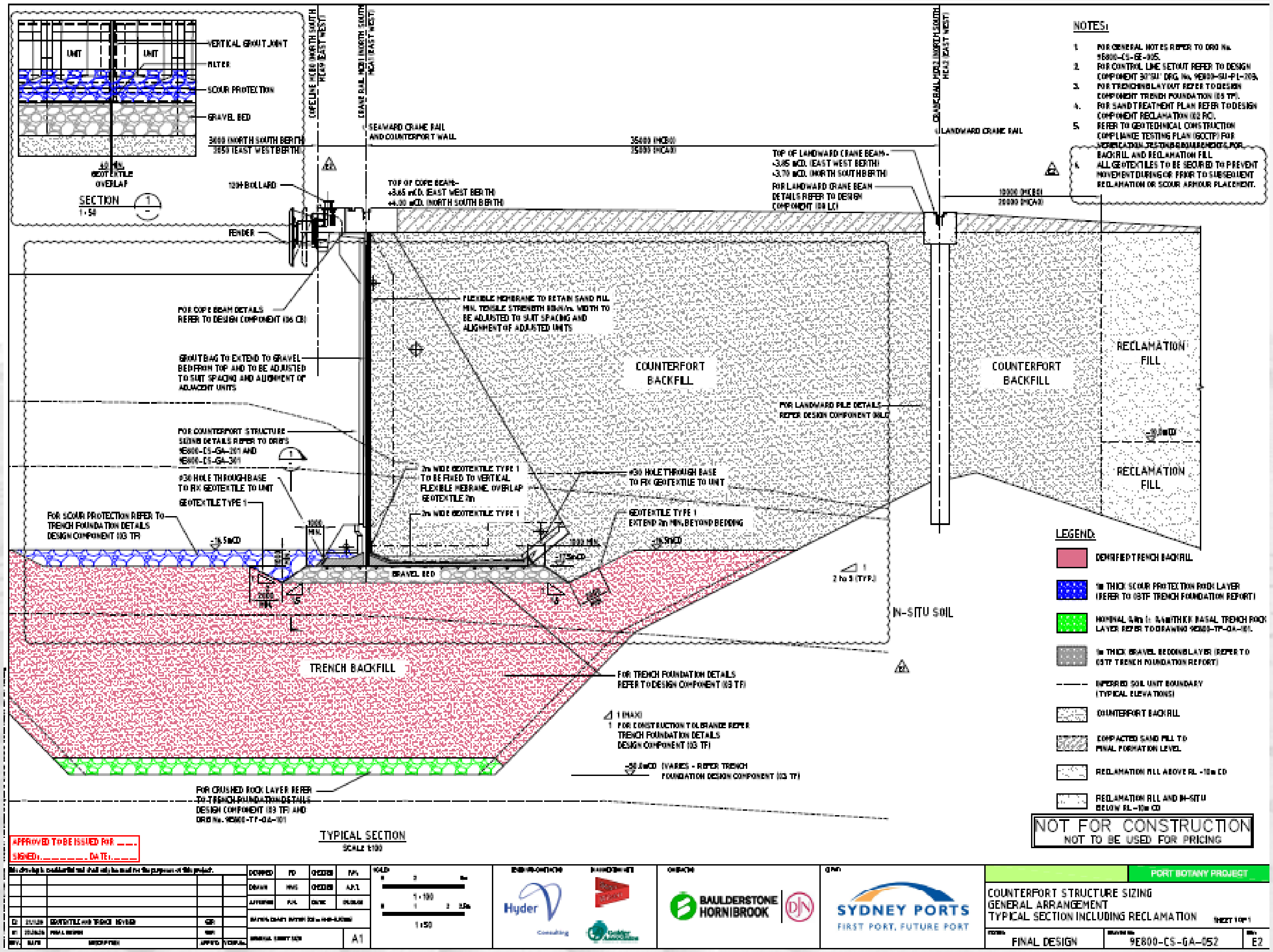
PORT BOTANY EXPANSION PROJECT

GROUND COMPACTION WORKS



PORT BOTANY EXPANSION PROJECT

GENERAL ARRAGEMENT COUNTERFORTS INCLUDING RECLAMATION



PORT BOTANY EXPANSION PROJECT

RESUME / QUANTITIES

PHASE	AREA (M2)	VOLUME (M3)	TECHNIQUE
EARLY WORKS	90.000	650.000	DYNAMIC COMPACTION / VIBRO COMPACTION
TRENCH FOUNDATIONS	64,000	800.000	OFFSHORE VIBROCOMPACTION
COUNTERFOURT BACKFILL	92.000	1.330.000	ONSHORE TANDEM VIBRO COMPACTION
NEW TERMINAL AREA	404.000	5.250.000	DYNAMIC COMPACTION
TOTAL	650.000	8.000.000	DC / VC

DYNAMIC COMPACTION

POUNDER WEIGHT 25 TON / 23 METERS HIGH DROP

5.0M X 5.0M GRID / 3 PHASES – 10 BLOWS

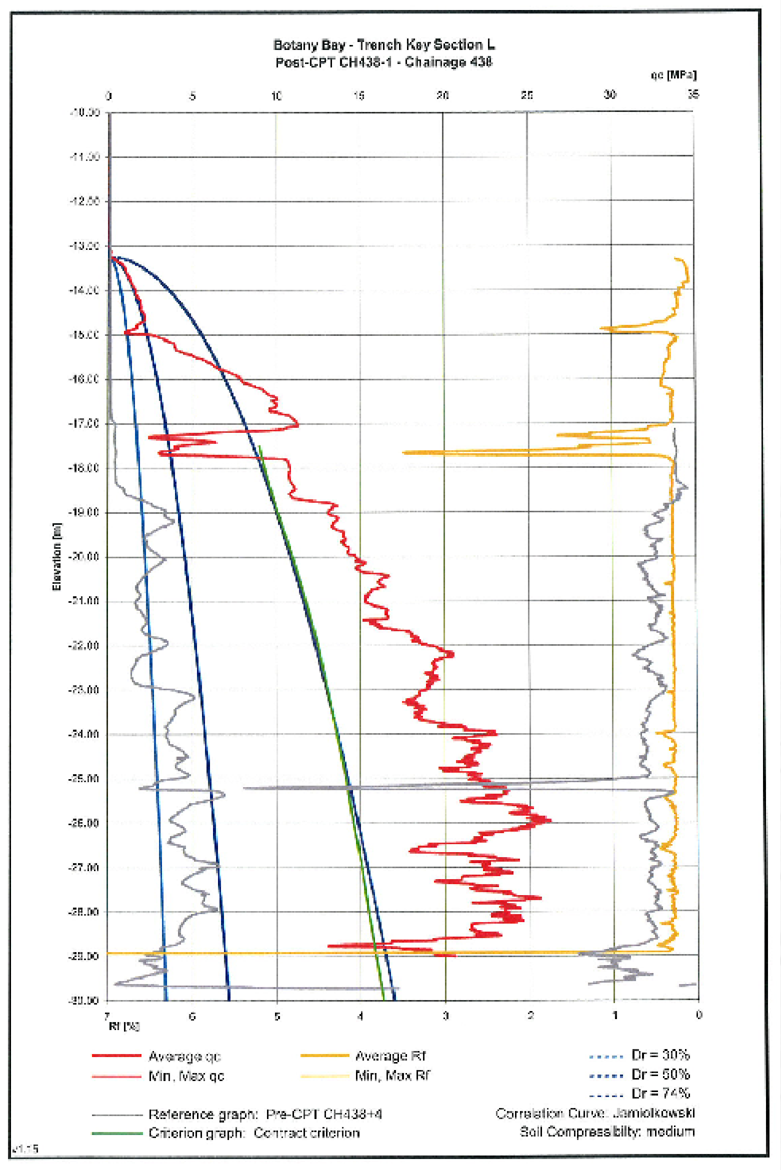
- **LOAD OUT WHARF – VIBRO COMPACTION V48**

UPLIFT STEPS 1.0M / 40 SEC EACH

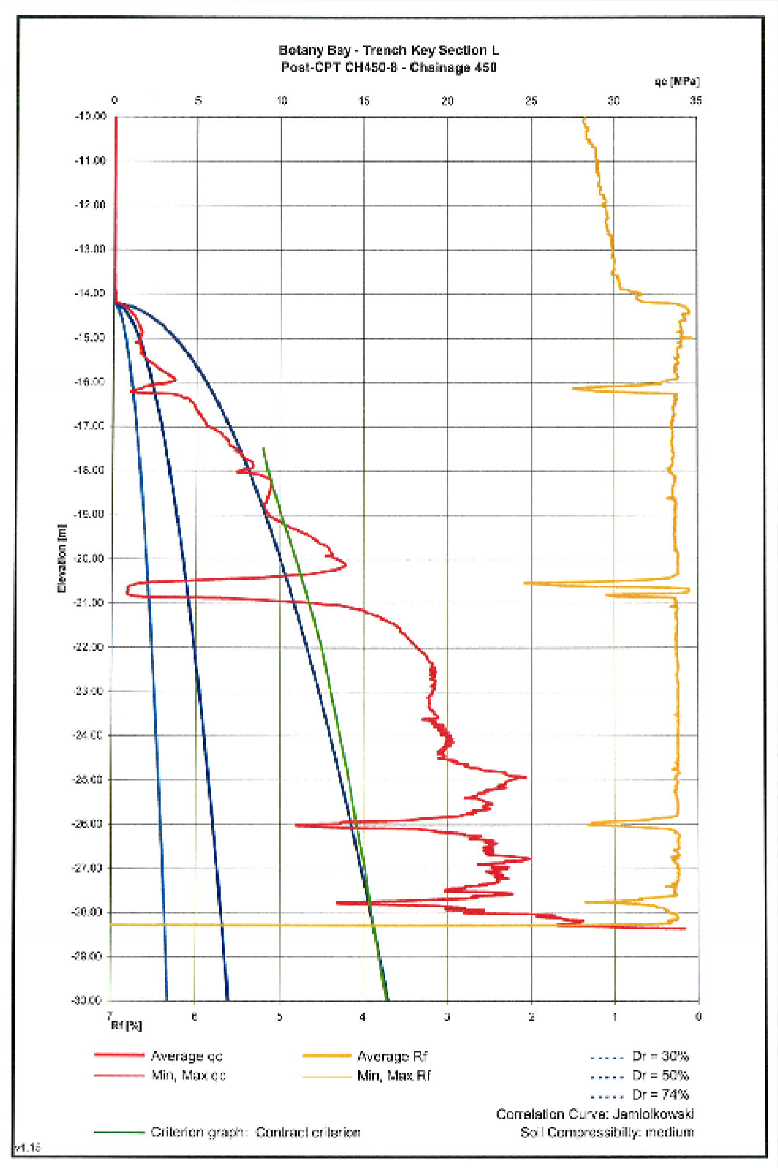
V48 REQUIRES WATER & AIR FOR COMPACTION

VIEW OF LOAD OUT WHARF – DC / VC WORKING

PORT BOTANY EXPANSION PROJECT

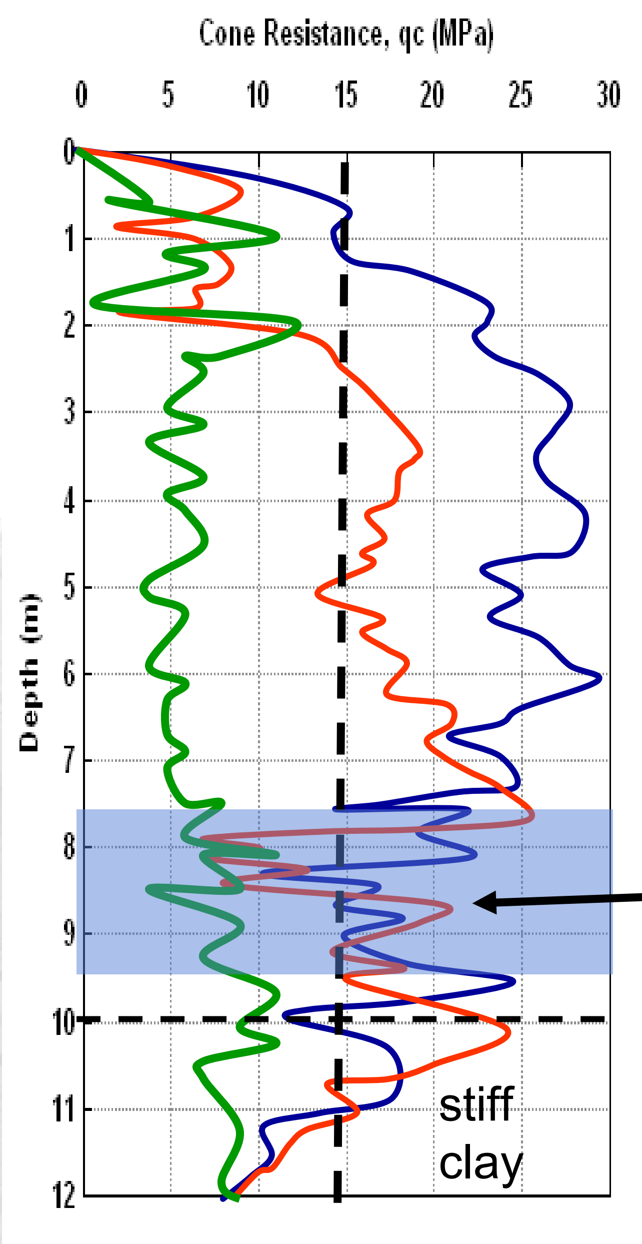


PORT BOTANY EXPANSION PROJECT



PERM – MASTER CLASS – NOV 2010

PORT BOTANY EXPANSION PROJECT



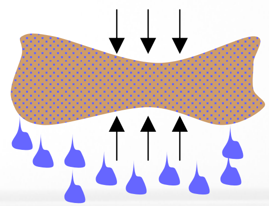
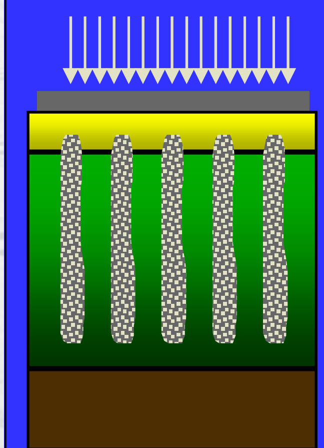
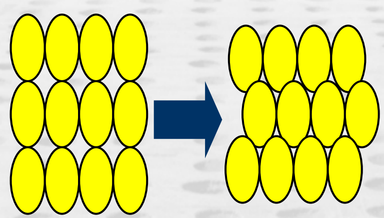
RESULTS

1. Except for the upper 50cm, the combination of VC and DC satisfied the q_c = 15 MPa (upper 0.5m requires surface roller compaction).

2. Enforced settlement:
   After VC – 47cm
   After DC – 27cm
   Total – 74 cm (~ 10% of treatment depth)

Compaction was less effective in this layer!

Soil Improvement Techniques

	Without added materials	**With added materials**
Cohesive soil **Peat , clay …**	1 Drainage 2 VAcuum 	4 Dynamic replacement 5 Stone columns 6 CMC 7 Jet Grouting 8 Cement Mixing 
Granular soil **Sand , fill**	3 Dynamic consolidation 4 Vibroflottation 	

Dynamic Replacement

CONCEPT

-Very soft to stiff soils

-Unsaturated soft clays

-Thickness of less than 6 meters

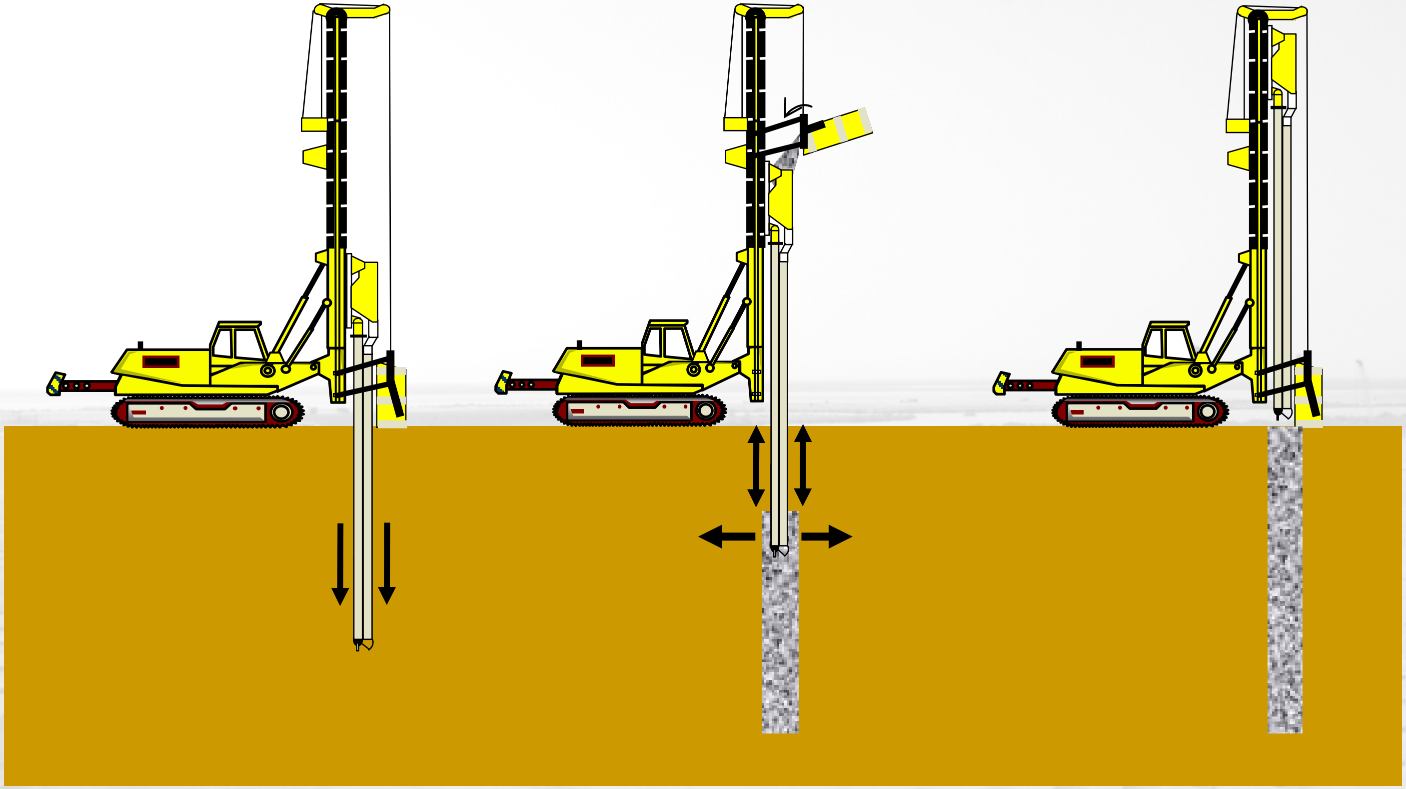
-Arching layer available

PARAMETERS

-C, $\varnothing$, μ, E_y of soil, column and arching layers, grid

-or P_L, E_P, μ of soil, column and arching layers, grid

Stone Columns – Bottom Feed



Vibrator penetration

Material feeding

Vibration of material during extraction

Principle of the technology - bottom feed with air tank

Stone Columns – Bottom Feed

Stone Columns bottom feed to 22 m depth

Stone Columns

CONCEPT

-Soft to stiff clays

-Thickness up to 25 meters

-Arching layer available

PARAMETERS

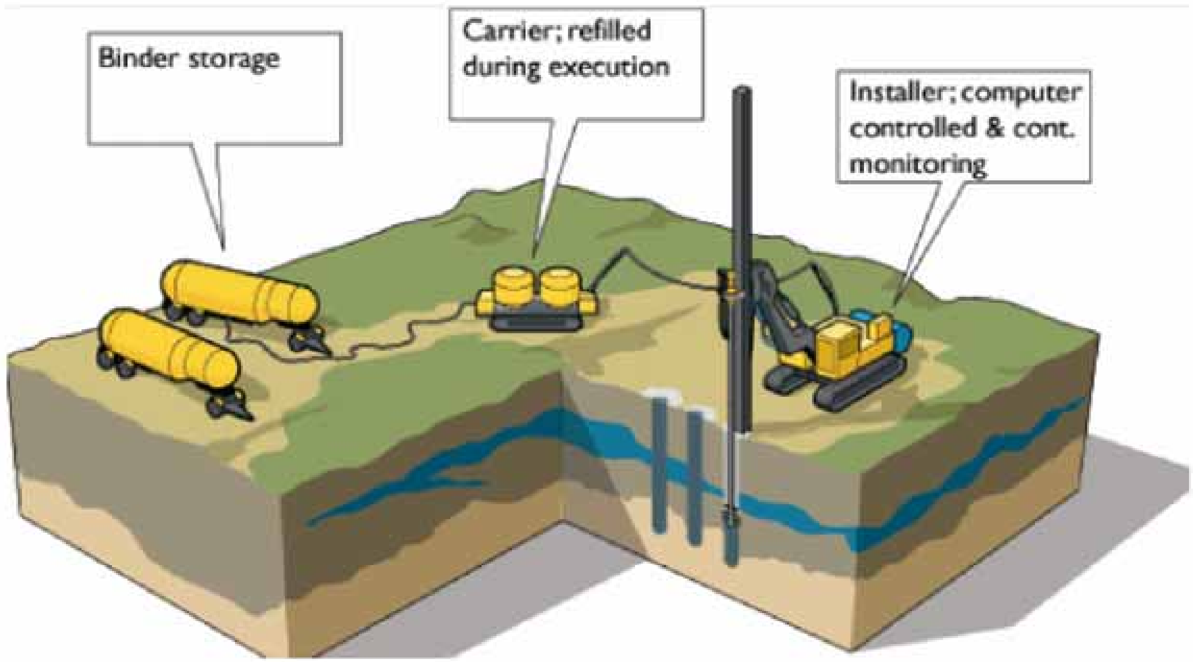
-C, ∅, μ, E_y of soil, column and arching layers, grid

-or P_L, E_P, μ of soil, column and arching layers, grid

CONCEPT

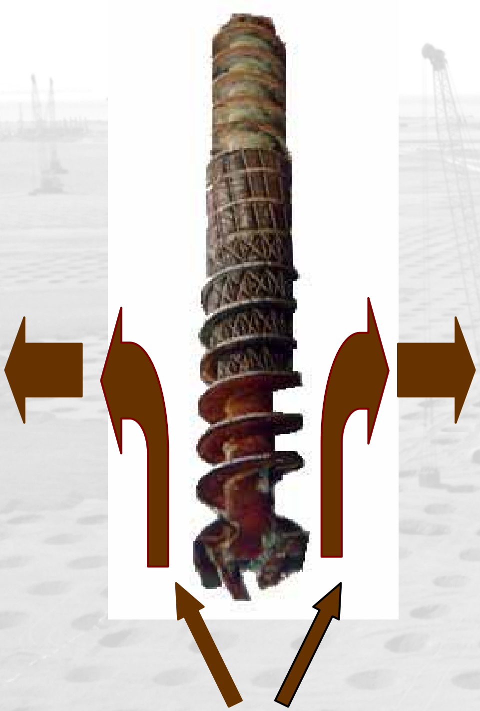
The MDM process (1)

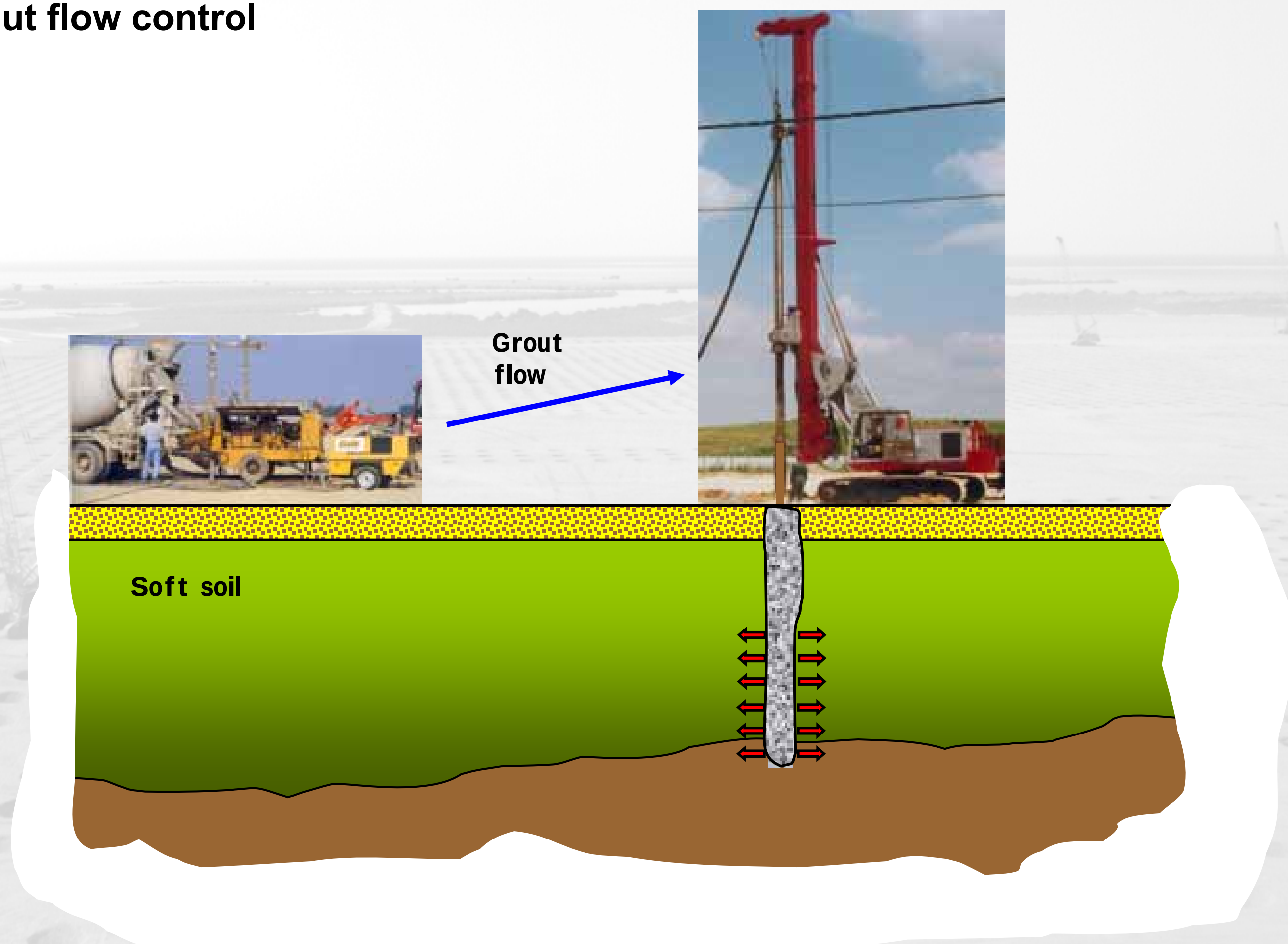
Site logistics



CMC – Execution

- Fleet of specilized equipment
  - Displacement auger => quasi no spoil
  - High torque and pull down
- Fully integrated grout flow control



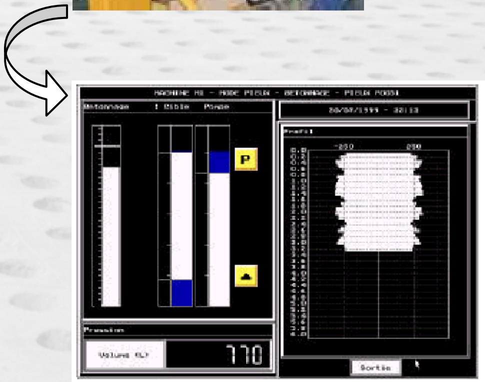


CMC – Typical Testing

- **Load testing on isolated CMC**
  - Checking of individual capacity,
  - Checking of adequate soil parameters taken into account.

- **Compression tests on material**
  - Checking of good grout resistance

- **Data recording system during execution**
  - Recording of drilling parameters => Checking of anchorage,
  - Recording of grouting parameters => No necking



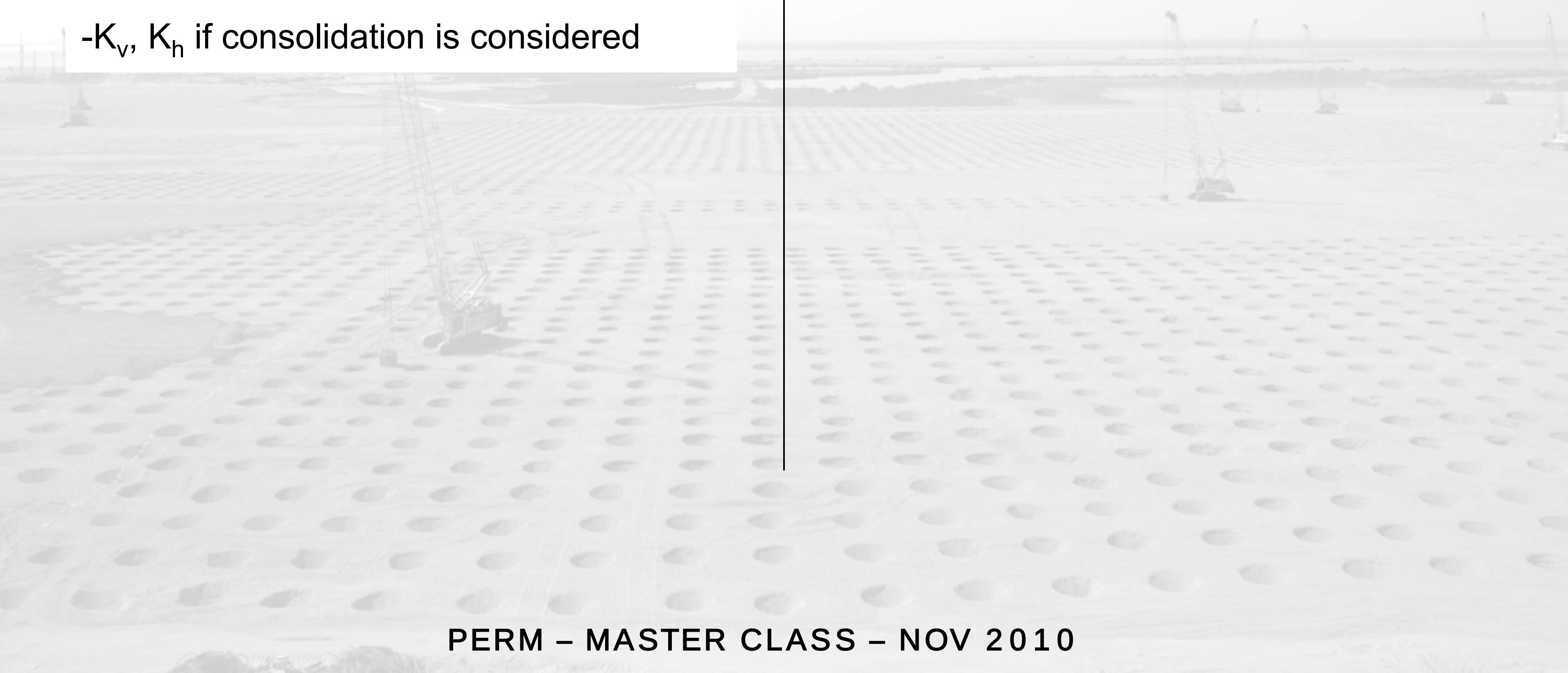
RIGID INCLUSIONS - PARAMETERS

SOIL

- C', $\varnothing'$, E_y, μ, γ, φ
- K_v, K_h if consolidation is considered

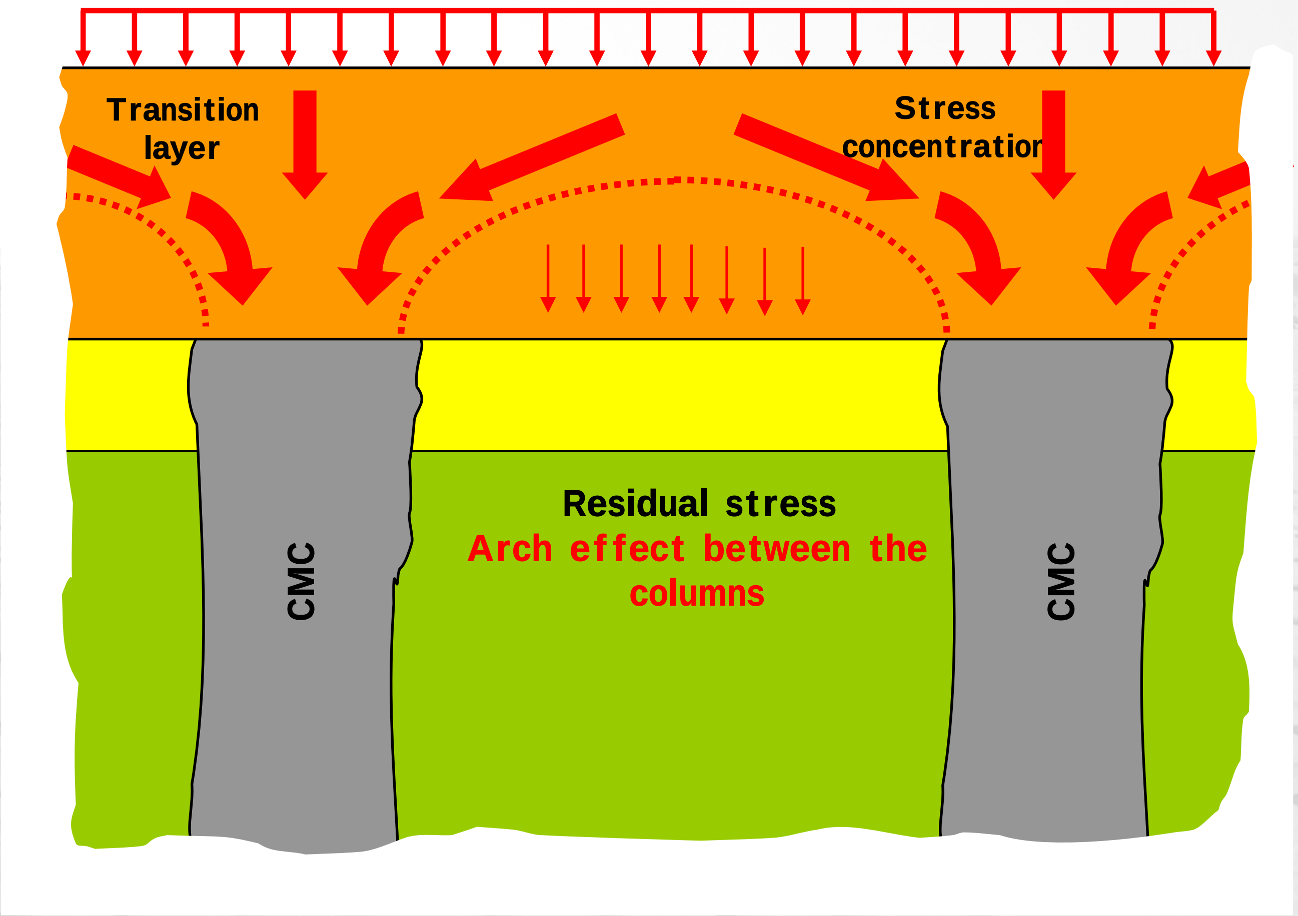
INCLUSION

- E_y, μ, γ, D (non porous medium)



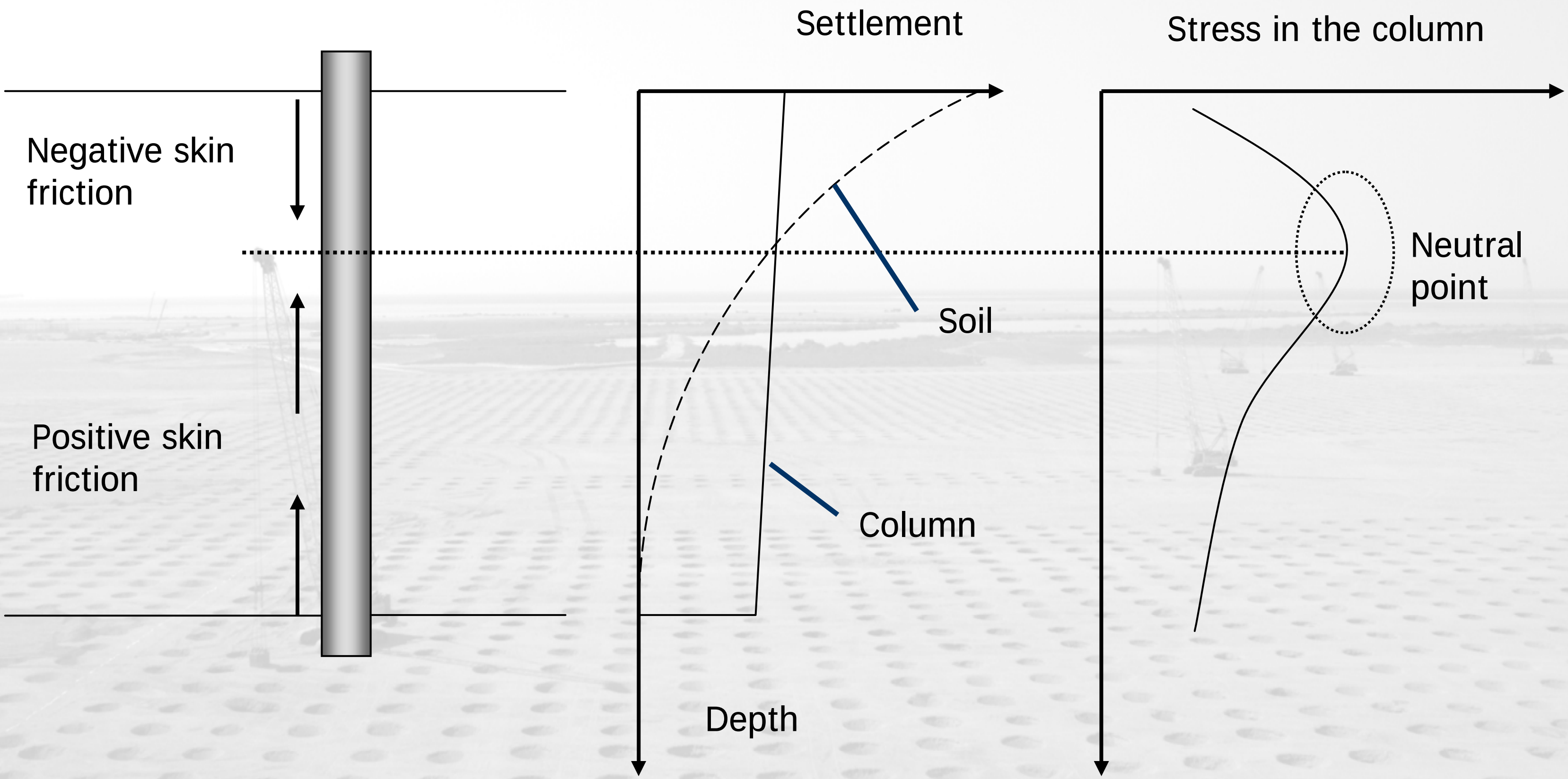
CMC Principle

- Create a composite material Soil + Rigid Inclusion (CMC) with:
  - Increased bearing capacity
  - Increased elastic modulus
- Transfer the load from structure to CMC network with a transition layer

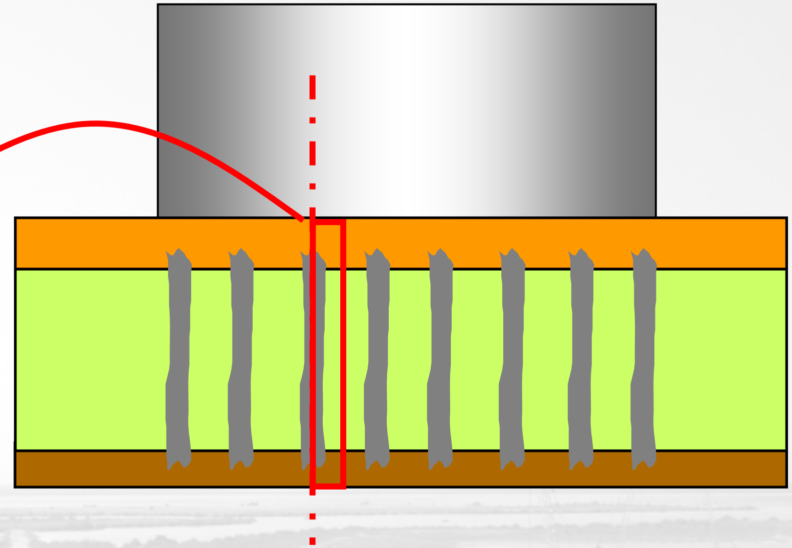


CMC - Basic behavior under uniform load

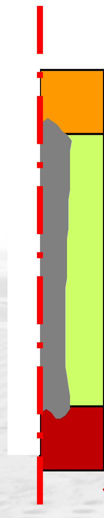
- **Negative skin friction allows to develop a good arching effect**



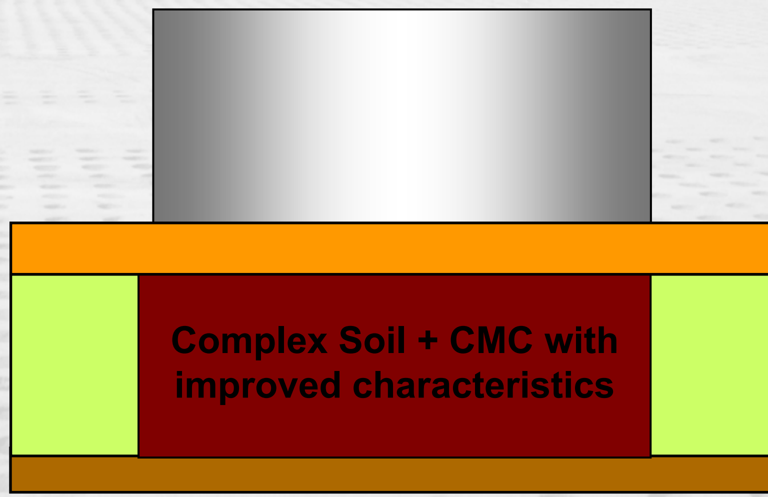
CMC Design - Principle



Axisymetric FEM calculation with one CMC and the soil => eq. Stiffness



Global axisymetric calculation by modelising the improved ground by material having an improved stiffness



CMC Design – Specific case of non vertical loading

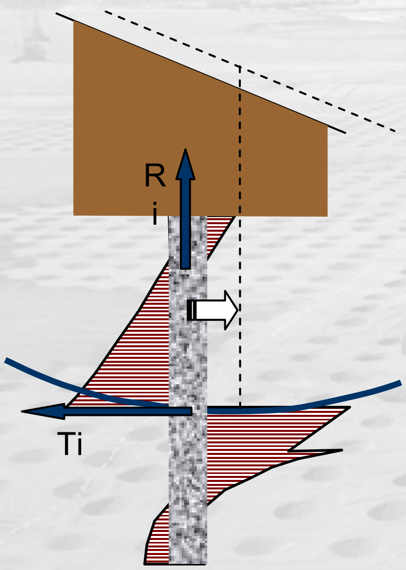
- ## Calculation principle

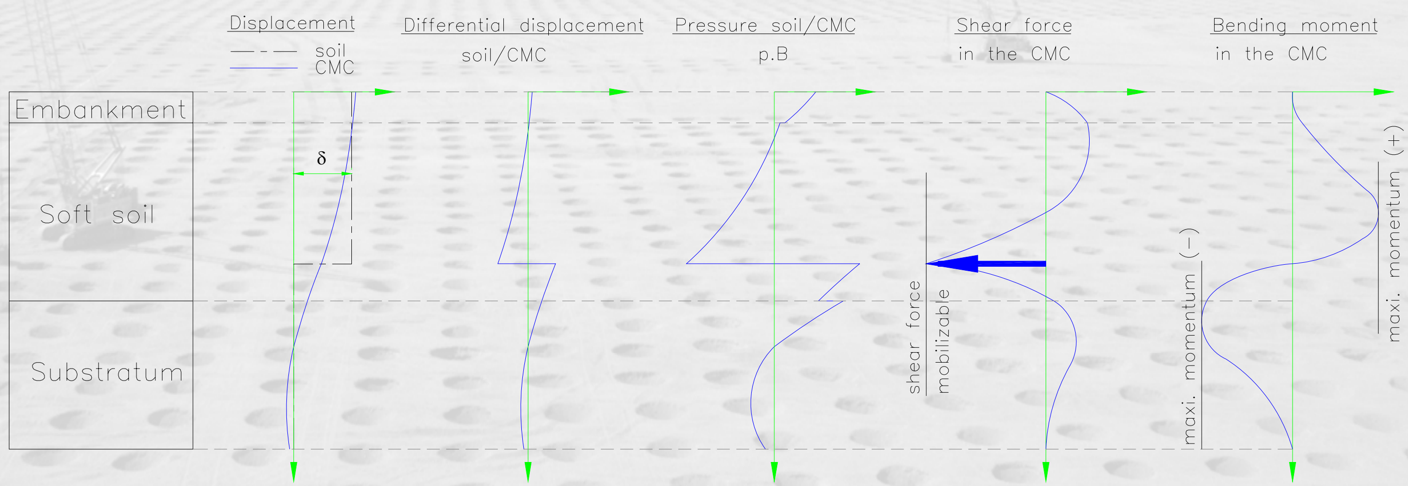
1/ Estimation of the vertical stress in the column (% of the embankment load),

2/ Thus maximum momentum so that M / N ≤ D / 8 (no traction in the mortar),

3/ Thus maximum shear force taken by the inclusion (similar to a pile to which a displacement is applied),

4/ Modeling of the CMC as nails working in compression + imposed shear force under TALREN software (or equivalent).





CMC Design – Benefits for the structure

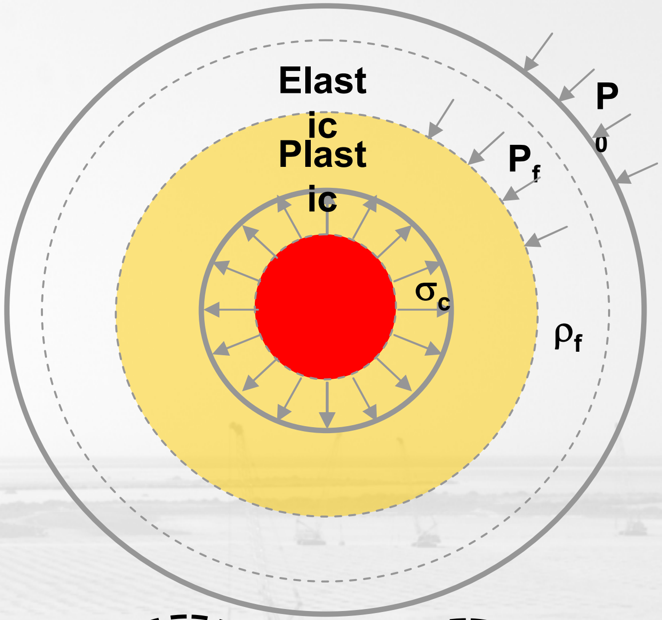
- **Structure shall be designed as if soil was of good quality**
  - Specialist contractor provides structural designer with bearing capacity, k, etc…

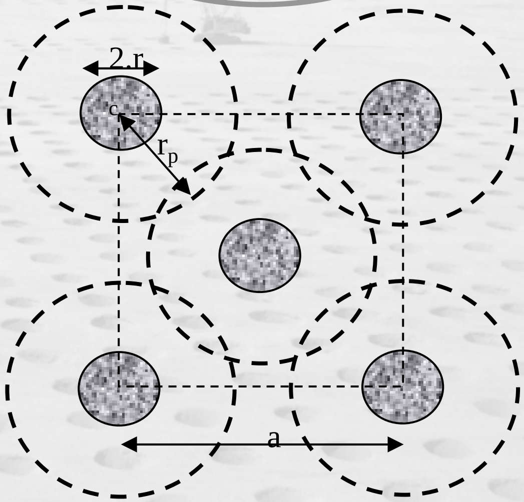
- **No connection between foundation and structure**
  - Structure is less complex to be designed,
  - No stiff connection, thus no increase under seismic analysis,
  - Structure very simple to be built: footings and slab on grade, no pile cap, thus benefit in terms of cost and speed of execution

New Developement - CMC Compaction - Principle

- **Aim of CMC CompactionDensify granular material to decrease liquefaction potential**

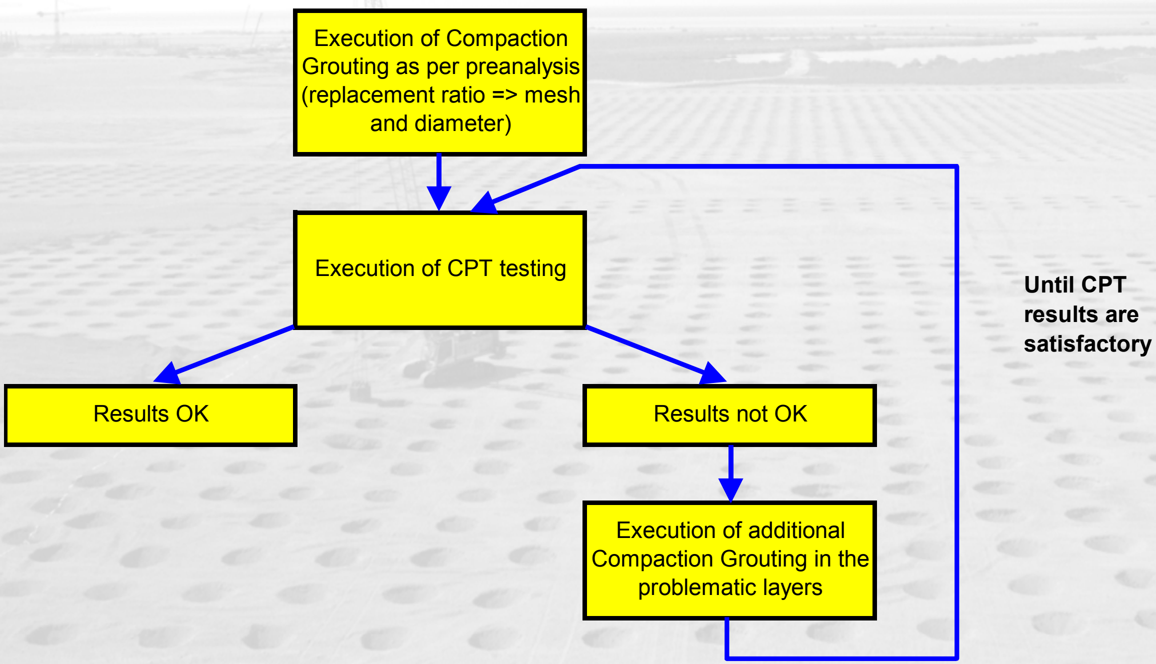
- **Method of densification**
  - Injected mortar used to displace and compact the soil around the injection point
  - Successive injection according to a regular grid induce a global compaction of the soil
  - Mesh and diameter designed so as to achieve a given replacement ratio

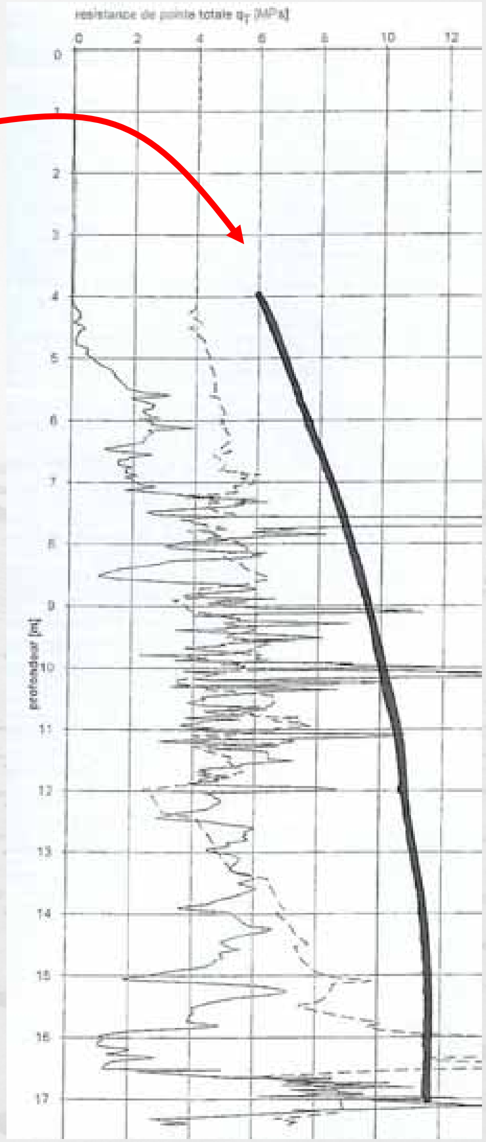




New Developement - CMC Compaction - Design

- **Principle: Execution and testing procedure**
  - **Seismic parameters (seism PGA, Magnitude) => <u>qc soil profile</u> to be achieved (Seed and Idriss methodology)**
  - **Estimation of Replacement ratio to achieve required qc**
  - **Execution of Works and testing by CPT**
  - **Additional grouting if necessary**



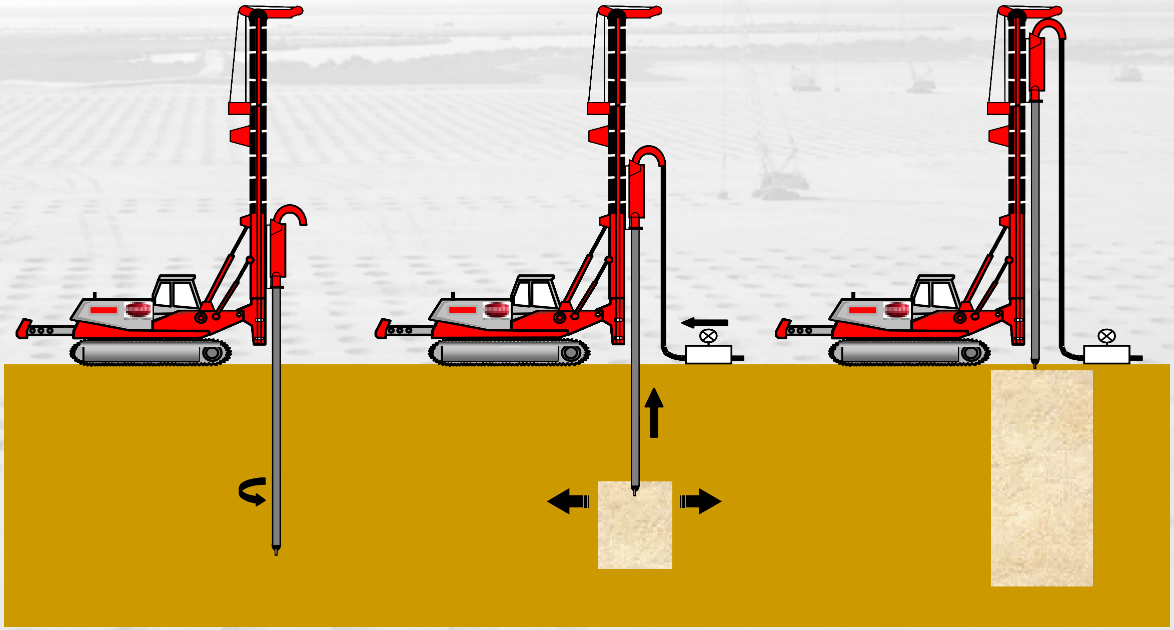


New Developement - CMC Compaction - Execution

- **Same type of equipment as for CMC**
  - Soil displacement rig and Pump,

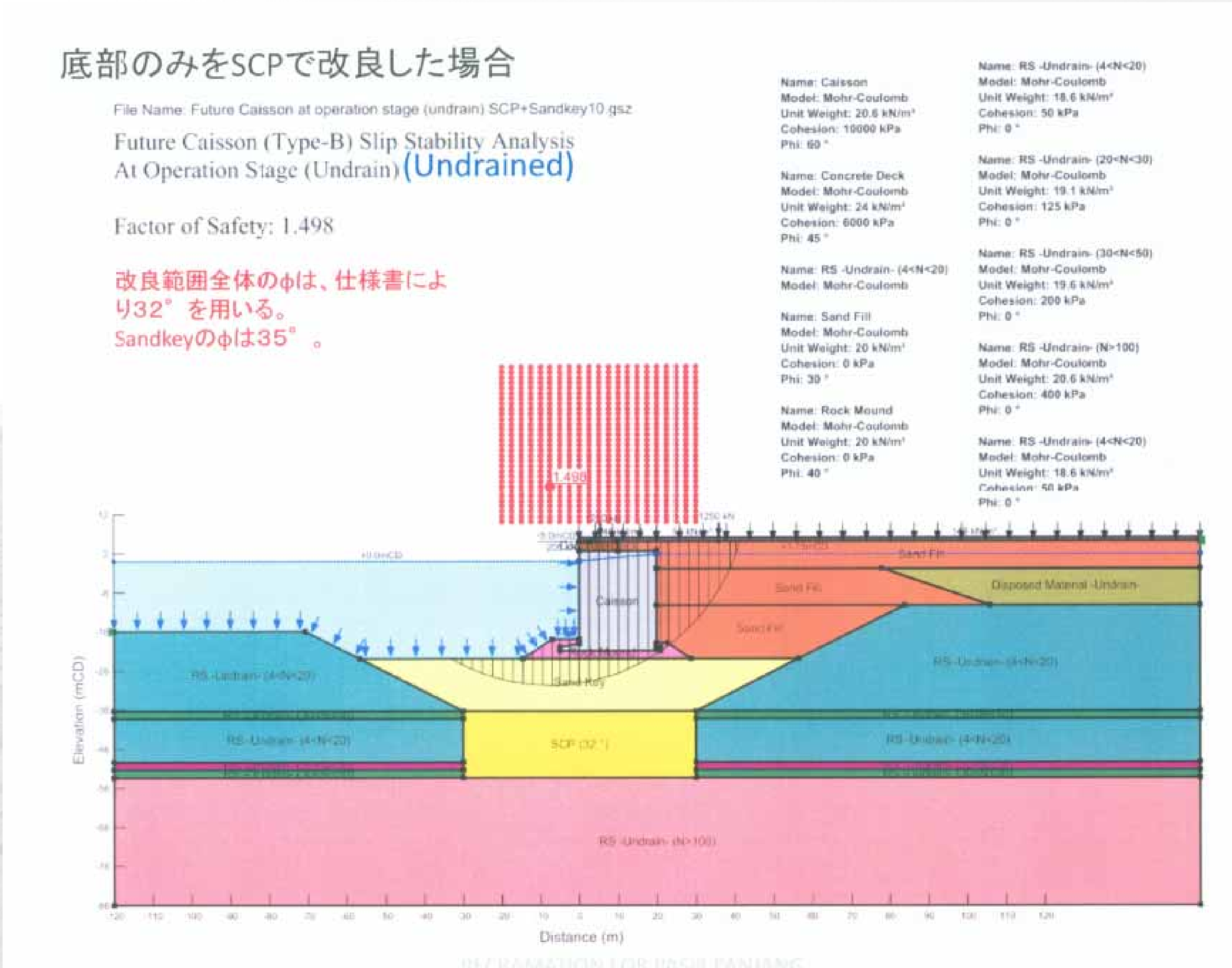
- **Key points**
  - Quality of grout (grain size distribution, workability, consistancy)
  - Injection speed and successive phases

- **Final Testing = CPT**

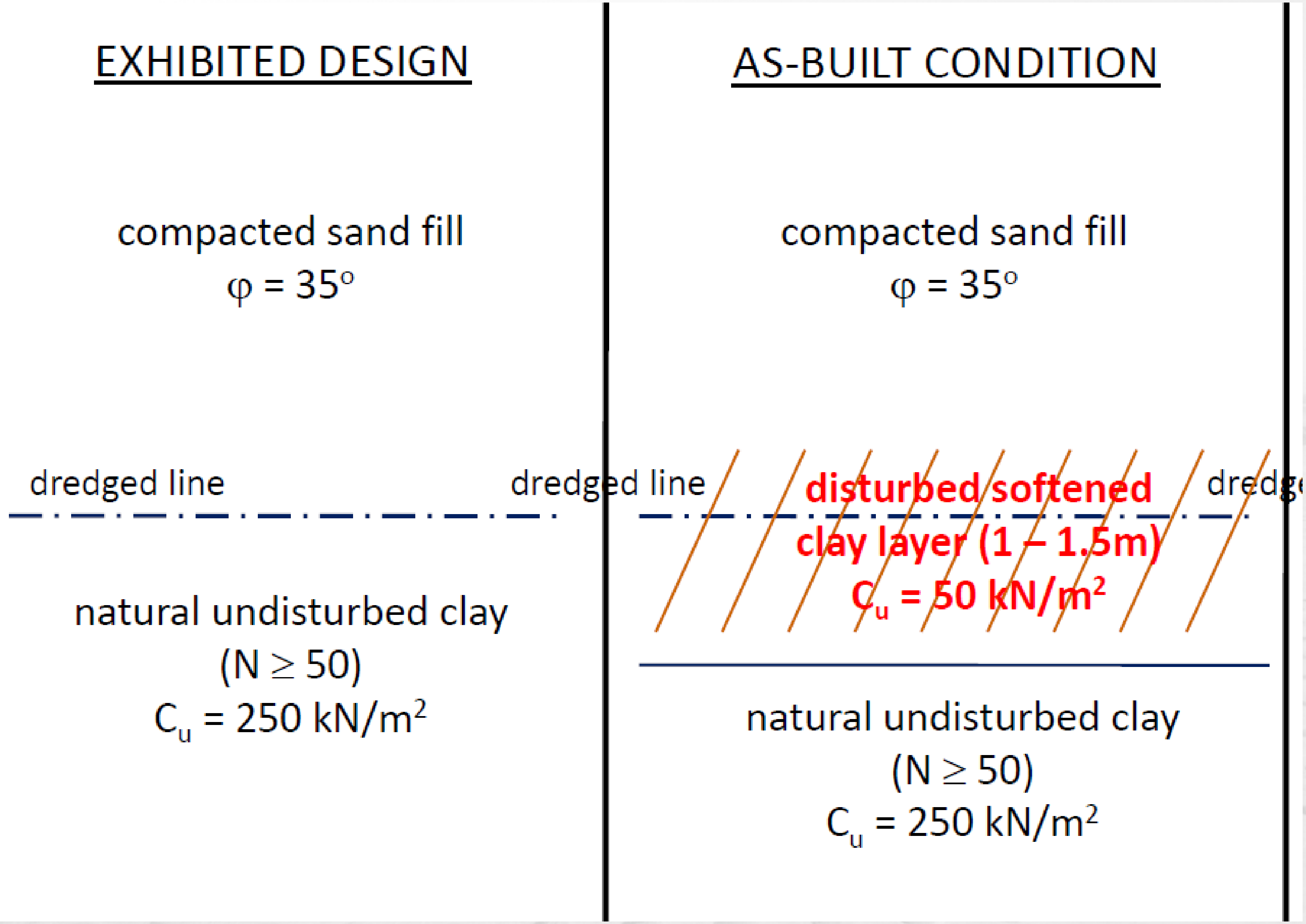


New Developement - CMC Compaction – Fos LNG Terminal

Future Caisson Stability Analysis



As built conditions

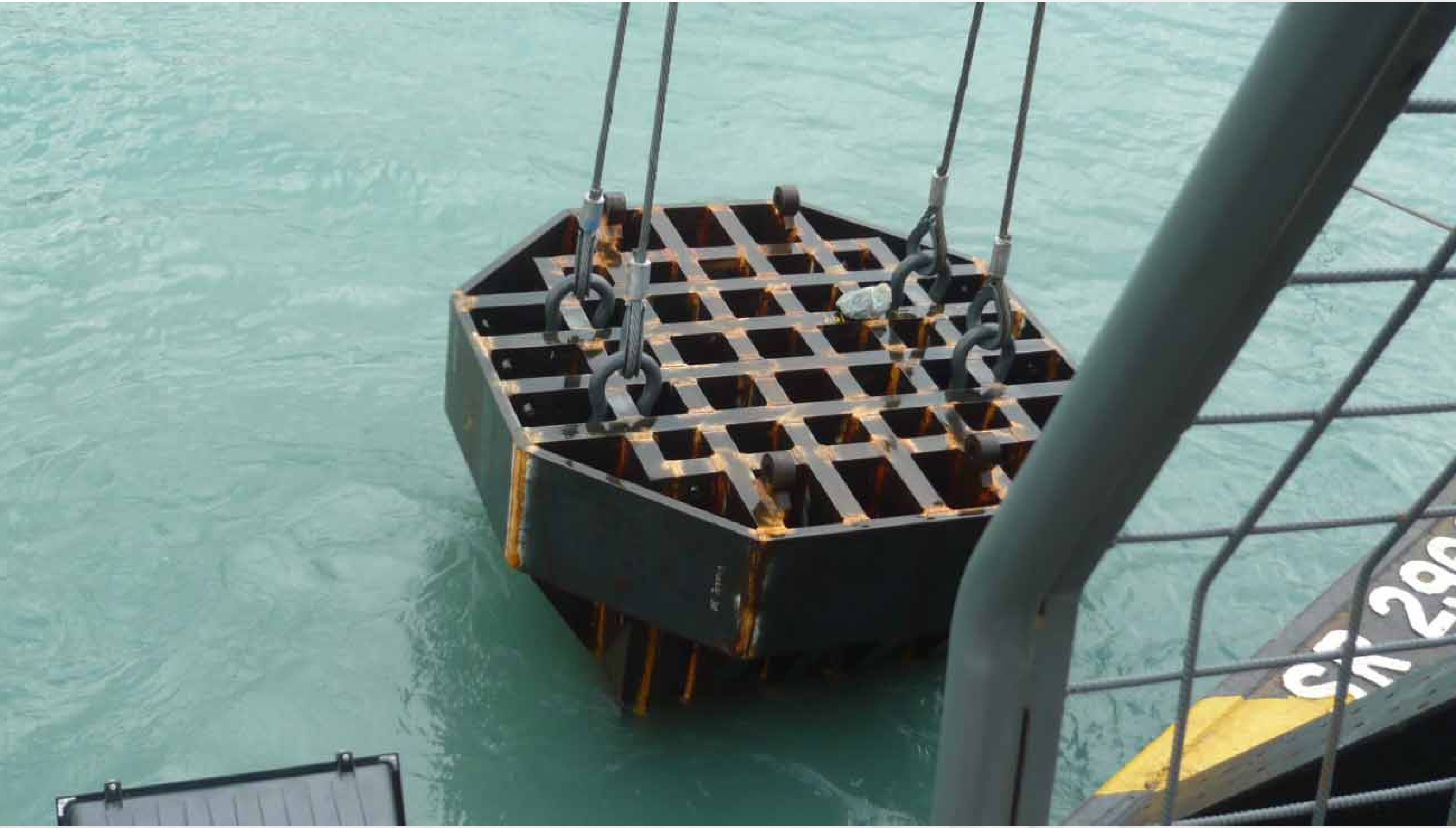


Proposed solution

Thickness	EXHIBITED DESIGN	PROPOSED SOLUTION	Thickness
	compacted sand fill $\varphi = 35^o$	compacted sand fill $\varphi = 35^o$	
1.3m	compacted sand fill $\varphi = 35^o$ $C = 0$	compacted rock mat ($\varphi = 45^o$ $C = 0$)	1.3m
	dredged line	dredged line	
1.5m	undisturbed clay $\varphi = 0^o$ $C_u = 250$ kN/m^2	15% rock ($\varphi = 45^o$) + 85% clay ($C_u = 80$ kPa)	1.5m
	natural undisturbed clay ($N \geq 50$) $\varphi = 0^o$ $C_u = 250$ kN/m^2	natural undisturbed clay ($N \geq 50$) $\varphi = 0^o$ $C_u = 250$ kN/m^2	

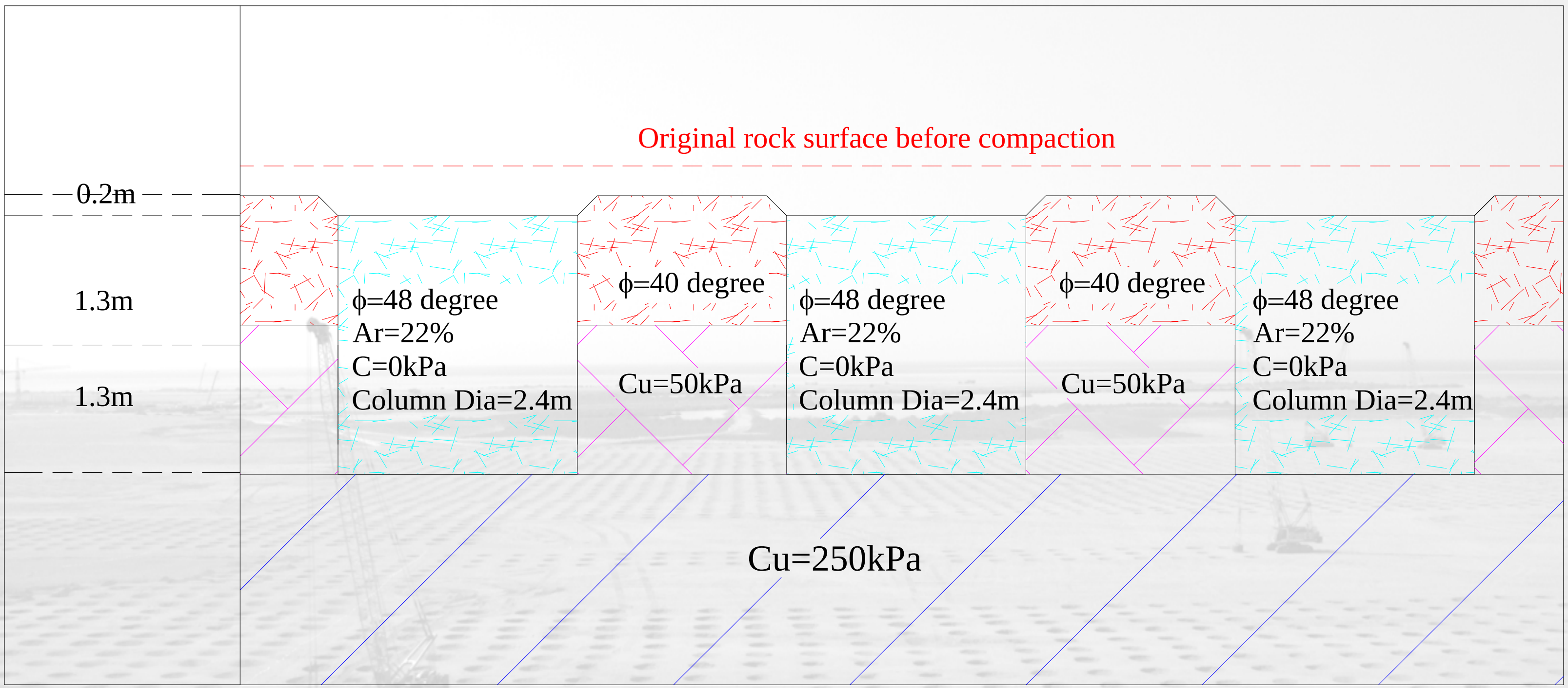
View of pounder construction

View of pounder ready to work



General SFT up

After compaction actual results



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THANK YOU

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